# TUCKER FREETBRARY RENOVATION PROJECT HENNIKER, NEW HAMPSHIRE

# BUDGET SET NOT FOR CONSTRUCTION

# JANUARY 17, 2020





**ENGINEER:** 

**SITE/CIVIL ENGINEER:** 

# CONSTRUCTION **MANAGER:**

# **DRAWING LIST:**

## Cover

## Structural:

S001	General Structur
S002	General Structur
S003	General Structur
S101	Foundation & E
S102	Second & Third
S103	Roof Framing P
S201	Steel Column Se
S301	Typical Founda
S302	Typical Slab Or
S303	Misc. Concrete
S304	Foundation Sect
S401	<b>Typical Framing</b>
S402	Framing Section
S501	Typical Wood F
S502	Typical Wood F
S601	Typical CFS De
S701	Typical Masonr

## Architectural:

A001	Code/Wall Types/I
A002	Rating Plan and Eg
A100	Sub-Basement
A101	First Floor Plan
A102	Second Floor Plan
A103	Third Floor Plan
A104	Roof Plan
A110	Reflected Ceiling I
A200	Elevations
A300	<b>Building Sections</b>
A400	Wall Sections
A402	Elevator Sheet
L001	Landscaping Plan

## **STRUCTURAL ENGINEER:**

# MECHANICAL/ELECTRICAL

- ural Notes ural Notes ural Notes E-Room Floor Framing Plans d Floor Framing Plans Plans Schedule & Steel Base Plate Details ation Details In Grade Details Details ctions ng Details ns Framing Details Framing Details etails ry Details
- bes/Project Summary l Egress

- ing Plans
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## Sheerr McCrystal Palson Architecture, Inc. THE CARRIAGE HOUSE 30 South Main St. Building Two Concord, NH 03301 603.228.8880, Fax 603.228.8881

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2020 JANUARY 17, DATE:

> BUDGE •• ISSUED LIBRARY FREE TUCKER

**PROJECT:** 

SET

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#### ABBREVIATIONS ABBREVIATIONS (cont'd) 2x = 2" NOMINAL THICK LUMBER RAD. = RADIUS REC. = RECOMMENDATION AB = ANCHOR BOLT REINF. = REINFORCING / REINFORCE(D) AFF = ABOVE FINISH FLOOR REQ'D. = REQUIRED ALT. = ALTERNATE REV. = REVISION ALUM. = ALUMINUM RF = ROOF APPROX. = APPROXIMATE RO = ROUGH OPENING ARCH. = ARCHITECTURAL RTU = ROOF TOP UNIT B/, B/O = BOTTOM OF . SCHD. = SCHEDULE BCX = BOTTOM CHORD EXTENSION SE = STRUCTURAL ENGINEER BJ = BAR JOIST SF = SQUARE FEET BLDG. = BUILDING SIM. = SIMILAR BM. = BEAM SPEC. = SPECIFICATION BOT. = BOTTOM STD. = STANDARD BP = BASE PLATE STIFF. = STIFFENER / STIFFEN(ED) BRG. = BEARING STL. = STEEL **BRP = BEARING PLATE** STRUCT. = STRUCTURAL BS = BRICK SHELF BTW = BETWEEN TBD = TO BE DETERMINED T/, T/O = TOP OF. CANT. = CANTILEVER TCX = TOP CHORD EXTENSION CFS = COLD FORMED STEEL THK. = THICK CIP = CAST IN PLACE TJ = TIE JOIST CJ = CONTROL JOINT T/O BS, TOBS = TOP OF BRICK SHELF CL. = CENTERLINE T/O STL, TOS = TOP OF STEEL CLR. = CLEAR T/O WALL, TOW = TOP OF WALL CMU = CONCRETE MASONRY UNIT TRANS. = TRANSVERSE COL. = COLUMN TYP. = TYPICAL CONC. = CONCRETE CONST. = CONSTRUCTION U/S = UNDERSIDE CONT. = CONTINUOUS UNO = UNLESS NOTED OTHERWISE COORD. = COORDINATE VB / VR = VAPOR BARRIER / RETARDER CTR. = CENTER VERT. = VERTICAL DBL = DOUBLE VIF = VERIFY IN FIELD Ø / DIA. = DIAMETER W/ = WITH DIM. = DIMENSION W/O = WITHOUT DIST. = DISTANCE WD. = WOOD DJ = DOUBLE JOIST WL. = WALL DK = DECK WK. PT. = WORK POINT DN. = DOWN WS. = WATERSTOP DWGS. = DRAWINGS WWF / WWM = WELDED WIRE FABRIC / MESH EA. = EACH EF = EACH FACE EIBC = EXISTING INTERNATIONAL BLDG. CODE EL. = ELEVATION ELEC. = ELECTRICAL ELEV. = ELEVATOR EMBED. = EMBEDMENT ENG = ENGINEER EOD = EDGE OF DECK EOR = ENGINEER OF RECORD EOS = EDGE OF SLAB EQ. = EQUAL ERV = ENERGY RECOVERY UNIT EW = EACH WAY E. / EX. / EXIST. = EXISTING EXP. = EXPANSION EXT. = EXTERIOR FFE = FINISHED FLOOR ELEVATION FIN. = FINISHED FLR. = FLOOR FNDN. = FOUNDATION FT. = FEET FTG. = FOOTING GA. = GAUGE GALV. = GALVANIZED GC = GENERAL CONTRACTOR GEOTECH. = GEOTECHNICAL HDG = HOT DIPPED GALVANIZED HORIZ. = HORIZONTAL HSS = HOLLOW STRUCTURAL SECTION IBC = INTERNATIONAL BLDG. CODE IF = INSIDE FACE IN. = INCH INT. = INTERIOR JNT. = JOINT JP = JOIST BEARING PLATE JST. = JOIST K = KIP LB = POUND LGM = LIGHT GAUGE METAL LLH = LONG LEG HORIZONTAL LLV = LONG LEG VERTICAL LONG. = LONGITUDINAL L.P. = LOW POINT LP = LEVELING PLATE LSL = LAMINATED STRAND LUMBER LVL = LAMINATED VENEER LUMBER MANUF. = MANUFACTURER MAX. = MAXIMUM MECH. = MECHANICAL MEP = MECHANICAL, ELECTRICAL, PLUMBING MIN. = MINIMUM ML = MASONRY LINTEL MO = MASONRY OPENING MPH = MILES PER HOUR MAS. / MSNRY. = MASONRY MTL. = METAL NIC = NOT IN CONTRACT/SCOPE # / No. = NUMBER NTS = NOT TO SCALE OC / o.c. = ON CENTER OF = OUTSIDE FACE OPNG. = OPENING OSB = ORIENTED STRAND BOARD PAF = POWDER ACTUATED FASTENER PC = PRECAST **PE = PROFESSIONAL ENGINEER** PEMB = PRE-ENGINEERED METAL BLDG. PL. = PLATE PLF = POUNDS PER LINEAR FOOT PRE-ENG. = PRE-ENGINEERED PSF = POUNDS PER SQUARE FOOT PSI = POUNDS PER SQUARE INCH PSL = PARALLAM STRAND LUMBER PT = PRESSURE TREATED

PWD. = PLYWOOD

#### GENERAL:

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14.

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1.

Structural drawings shall be used in conjunction with the architectural, mechanical, electrical and shop drawings, and specifications.

2. Unless otherwise noted, sections, details, notes, materials, and methods shown on any drawings are to be considered typical for all similar conditions.

3. In the event of a conflict between plans, specifications, and details, the Structural Engineer shall be notified immediately for clarification.

4. Due to minimal selective demolition, the existing framing conditions are not fully defined and will require field verification.

The general contractor must field verify and review all existing framing for coordination with newly detailed structural assemblies. For the purpose of preparing these drawings, the engineer has assumed all walls and framing are plumb, level, align vertically and horizontally and all members are sound. Depending on conditions encountered, it may be necessary to modify the design. The G.C. must notify the Structural Engineer (SE) of varying conditions prior to beginning construction.]

5. All dimensions, elevations, and conditions must be verified in the field by the Contractor. Any discrepancies between these drawings and as-built conditions shall be brought to the attention of the Structural Engineer before proceeding with any work.

6. The structure has been designed to be self-supporting and stable after the work shown on these drawings has been completed. The Contractor shall be responsible for the stability of the structure prior to the completion of work including but not limited to, jobsite safety, all shoring, bracing, erection methods, erection sequence, and forms required during construction. Temporary supports required for stability during all intermediate stages of construction shall be designed, furnished, and installed by the Contractor.

7. The Contractor shall provide and maintain shoring and bracing supports as required to preserve stability and prevent movement, settlement, or collapse of adjacent construction to remain.

8. All shoring and bracing shall be designed and certified by a professional engineer licensed in the jurisdiction of the project. Submittals to the Structural Engineer of all shoring and bracing for review and approval are required prior to the start of construction.

A complete concrete placement schedule shall be submitted to the Structural Engineer and a stamped acceptance received before any concrete placement can be made.

Shop drawings shall be submitted to the Structural Engineer (see each section for specific items and requirements). Fabrication shall not proceed until a satisfactory review is received, the Contractor is proceeding at their own risk if failure to do so. Erection shall be executed from final reviewed shop drawings only.

Items noted on drawings as "by others" or "designated for design by others" indicates design and supply of structural items not by TFM. These items are a designated design item that shall be submitted for approval. See Deferred Submittals.

12. Deferred submittals shall be submitted to the Structural Engineer for steel connection design (stamped), steel stair design (stamped), and CFSF curtain wall design (stamped).

These plans were prepared under the supervision of a licensed professional engineer. TFMoran Inc. assumes no liability as a result of any changes or non-conformance with these plans except upon the written approval of the Engineer of Record.

TFMoran Inc. assumes no liability for work performed without an acceptable program of testing and inspection as approved by the Engineer of Record.

Reproduction of structural drawings for shop drawings is not permitted. Electronic drawing files will not be provided to the Contractor unless a transfer agreement has been completed between the Structural Engineer and the Contractor.

16. All work shall comply with the building codes referenced on these drawings.

Do not scale drawings. Contact the Architect or Structural Engineer for dimensions not specifically shown.

#### CODE:

[2015 International Building Code as amended, altered, or deleted by the provisions of the New Hampshire State Building Code.]

## <u>DESIGN LOADS:</u>

MINIMUM UNIFORM LIVE LOADS AND MINIMUM CONCENTRATED LIVE LOADS: OCCUPANCY or USE UNIFORM CONCENTRATED

OCCUPANCY or USE	UNIFORM	CONCENTRATED			
Library					
Corridors above first floor:	80 psf	1000 lb.			
Reading rooms:	60 psf	1000 lb.			
Stack rooms:	150 psf	1000 lb.			
Office Buildings					
Lobbies and first floor corridors:	100 psf	2000 lb.			
Offices:	50 psf	2000 lb.			
Partitions:	15 psf (for l	15 psf (for Live Loads < 80 psf)			

Stairs and Exits: 100 psf 300 lb. on tread

[Live load has been reduced on girders, columns, and footings in accordance with the building code.]

 CONCENTRATED FLOOR LOADS: If listed above, the concentrated load shall be used to determine the greatest load effect. Unless otherwise specified, the indicated concentration shall be assumed to be uniformly distributed over an area of 2.5 feet square and located to produce the max. load effects.

3. ROOF SNOW LOAD: Risk Category: Ground Snow Load, Pg : 80 psf [at 1000 ft] 0.01\*(1000-450)\*2.1=11.55 psf] Allowed Reduction per ERDC/CRREL TR-02-6: Ground Snow Load per ERDC/CRREL TR-02-6: 68.45 psf at 450 ft] Snow Load Importance Factor, Is: 10 Snow Exposure Factor, Ce: 1.0 Thermal Factor, Ct: 1.10 Flat Roof Snow Load, Pf: Per ASCE-7 Drifting, sliding, and unbalanced snow loads: Drift Surcharge Load(s), Pd: 33.9 psf [or] [See plan(s) / diagram] Width of snow drift(s), w: 5.92 ft [or] [See plan(s) / diagram] Rain loads: Per ASCE-7 Roof live load: 20 psf MIN 4. DEAD LOAD: Roof dead load: 20 psf Truss Top Chord / Bottom Chord: 10 psf (each) Solar Array Allowance: 5 psf]

100 psf

50 psf

10 psf / inch

Roof Garden: Supported / Elevated Floor dead load: Design of floor framing members: Gypcrete

5. WIND DESIGN DATA:

0.				
		etermined using ASCE-7[ M	•	
		velope Procedure - Simplifi		
		implified][ Directional Proce	dure - Analyti	cal].
	Risk Category:		II	
	Ultimate Wind Speed (3	second gust), Vult:	115 mph	
	Wind Exposure Category	':	В	
	Internal Pressure Coeffic		0.18	
	Components and Claddir	ng Design Wind Pressure:		
	Zone Per ASCE-7	MAX Positive (20 sf)	MA	<u>X Negative (20 sf)</u>
	1	16.0 psf		32.1 psf
	2	16.0 psf		37.6 psf
	3	16.0 psf		63.6 psf
	4	25.5 psf		27.8 psf
	5	25.5 psf		33.4 psf
6.	EARTHQUAKE DESIGN DA	TA:		
	Risk Category:		II	
	Seismic Importance Fact	tor, le:	1.0	
	0.2s Mapped Spectral Re	esponse Acceleration, Ss:	0.25	59g
	1.0s Mapped Spectral Re	esponse Acceleration, S1:	0.08	32g
	0.2s Spectral Response	Coefficient, Sds:	02	75g
	1.0s Spectral Response	Coefficient, Sd1:	0.13	31g
	Site Class:		D	
	Seismic Design Category	/:	В	
	Basic Seismic-Force-Res	sisting System:		ctural steel systems not
				cifically designed for seismic
				stance.
	Analysis Procedure:		•	iivalent Lateral Force
	Response Modification F		3	
	Seismic Response Coeff	icient, Cs:	0.09	92

Response Modification Factor, R: Seismic Response Coefficient, Cs: Deflection Amplification Factor, Cd: Design Base Shear, V:

Earthquake Design for Existing Buildings: Not required since the proposed additions/alterations do not increase the force in any structural element by more than 5 percent nor do they decrease the strength of any structural element to less than required by the building code for new structures.

7. Other Loads: Elevator Hoist Beam:

6000 lbs

0.092W kips

## PLAN SYMBOL LEGEND

1		INDICATES SLAB OVER-POUR, TYPICAL AT EXTERIOR DOOR, -SEE TYPICAL OVER-POUR DETAIL
2	SF	INDICATES STEPPED FOOTING, SEE "TYPICAL STEPPED FOOTING DETAIL" ON "TYPICAL FOUNDATION DETAILS" SHEET.
3	CJ	INDICATES CONTROL JOINT, SEE "TYPICAL SLAB-ON- GRADE DETAILS" ON "TYPICAL FOUNDATION DETAILS" 
4		INDICATES STEP IN SLAB.
5		INDICATES CONCRETE MASONRY UNIT (CMU) WALL.
7	F-# OR 	INDICATES FLOOR/ ROOF DECK SPAN DIRECTION AND TYPE. SEE PLAN NOTES FOR CONSTRUCTION INFO.
8		INDICATES DOWNWARD SLOPE DIRECTION AND PITCH.
11		INDICATES OVERBUILD TRUSS FRAMING (OR RAFTER FRAMING IF APPLICABLE, SEE PLAN NOTES), TRUSSES (OR RAFTERS) BELOW THE OVERBUILD MUST BE FULLY SHEATHED. ISOLATED OPENINGS IN SHEATHING MAY BE ALLOWED WITH THE ENGINEER'S APPROVAL.
12		INDICATES STEEL MOMENT FRAME, STUB CANTILEVER, OR SPLIT CANTILEVER. SEE PLAN FOR INFO.
13	#	INDICATES BRACED FRAME ELEVATION.

F	0	U	N	D	A٦	<b>[</b> ]	<u>0</u>	Ν	S	•
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- . Foundations have been designed to consist of continuous and spread footings bearing on inorganic, undisturbed natural soil or compacted structural fill having an assumed allowable bearing pressure of 3000 pounds per square foot. The contractor is responsible for verifying these subsurface conditions, failure to do so will result in a disclaimer of responsibility by the Structural Engineer.
- 2. Subgrade exploration has not been performed, the Structural Engineer makes no representations concerning the suitability of any soil or ledge material, nor the absence of deleterious materials, either naturally occurring or formerly buried.
- 3. Structural fill shall be granular material meeting the following gradation requirements:

SIEVE SIZE	<u>% PASSING BY WEIGHT</u>	SIEVE SIZE	% PASSING BY WEIGHT
8"	100	3/4"	45-95
6"		No. 4	30-90
3"	70-100	No. 10	25-80
2"		No. 40	10-50
1-1/2"		No. 200	0-12
1"			

A soils testing lab shall test all material proposed for structural fill for classification according to ASTM D2487 and for laboratory compaction curve according to ASTM D1557. In addition, in-place soils shall be tested for compaction to a minimum 95% of its maximum density at or near optimum moisture.

- 4. Unless otherwise noted, foundations shall be centered under supported members.
- 5. The bottom of perimeter and exterior foundations not on solid rock shall be at least [4'-0"] below finished grade.
- Keep foundation excavations free of water at all times. Protect all soil surrounding and under footings from freezing and frost action during the course of construction.
- 7. Bottom of excavations shall be reviewed by the Structural or Geotechnical Engineer prior to the placement of concrete.
- 8. Provide formwork for all footings, walls, and piers. Unless otherwise noted, earth formed foundations are not allowed.
- 9. Place backfill simultaneously on both sides of foundation walls to the grades indicated. Do not backfill or temporarily brace walls with uneven backfill until the floor slab at the top of the wall has been poured and/ or the concrete has attained 75% of its design compressive strength.
- 10. Provide 3/4" maximum aggregate within 12" of slabs on grade, unless otherwise noted by Geotechnical Engineer.
- 11. The bottom three (3) inches of footing excavations shall be finished with smooth-edged bucket or by hand shovel.
- 12. Use lean concrete (f'c = 1,500 psi) or structural fill for over-excavation of footings.
- 13. Refer to site, plumbing, mechanical, and electrical drawings for location of pipes and underslab conduit. Provide pipe sleeves for all pipe penetrations at foundation walls.
- 14. The G.C. shall identify all below grade utilities prior to commencing excavation activities.
- 15. Submittals to the Structural Engineer and Geotechnical Engineer are required for structural fill material, signed and sealed by the professional engineer, registered in the state of the project's construction, responsible for their preparation.

#### CONCRETE:

- 1. All concrete work shall conform to the requirements of ACI 301 "Specifications for Structural Concrete" and ACI 318 "Building Code Requirements for Structural Concrete".
- 2. Concrete shall be a mix designed for ultimate strength in accordance with ACI 211.1 to achieve the following minimum 28-day compressive strengths:
- [Foundation Footings, Walls, Columns, Piers, Grade Beams, and Pile Caps: 3,000 psi, Normal Weight Max Slump (without plant added water reducer) = 4" +/- 1"
- Max Slump (with plant added water reducer) = 4" to 6" Max W/C Ratio = 0.55 Air Entrainment = 6% +/- 1%]
- [Retaining Walls (Including Footings): 3,000 psi, Normal Weight Max Slump (without plant added water reducer) = 4" +/- 1" Max Slump (with plant added water reducer) = 4" to 6" Max W/C Ratio = 0.50 Air Entrainment = 6% +/- 1%]
- [Interior Slabs on Grade and Housekeeping Pads: 3,500 psi, Normal Weight Max Slump (without plant added water reducer) = 4" Max Slump (with plant added water reducer) = 4" to 6" Max W/C Ratio = 0.50 Do not use air entrainment admixture]
- [Exterior Site Structures, Outdoor Stairs, and Slabs: 4,000 psi, Normal Weight Max Slump (without plant added water reducer) = 3" Max Slump (with plant added water reducer) = 4" to 6" Max W/C Ratio = 0.45 Air Entrainment = 6% +/- 1%]
- 3. Concrete shall conform to the following:
- Cement: Portland cement type I/II ASTM C150 Fly Ash: ASTM C618 Class C 20% to 35% or Class F 15% to 25% Ground granulated blast-furnace slag: ASTM C989 50% maximum
- Course aggregate: [ASTM C33 3/4" (Size No.67) for normal weight or ASTM C330 3/4" (Size No.67) for light weight] Fine aggregate: [ASTM C33 3/8" (Size No.8) for normal weight or ASTM C330 3/8" (Size No.8) for light weight]
- 4. Concrete shall be placed under the supervision of an American Concrete Institute (ACI) qualified testing agency.
- 5. Concrete shall not be cast in water or on frozen ground.
- 6. Mechanically vibrate and consolidate freshly cast concrete around reinforcing bars and against form surfaced to prevent the formation of air or stone pockets, honeycombing, pitting, or planes of weakness. Do not over vibrate such that aggregate separation occurs.

TABLE OF CONTENTS	
NAME	#
GENERAL STRUCTURAL NOTES	S001
GENERAL STRUCTURAL NOTES	S002
GENERAL STRUCTURAL NOTES	S003
FOUNDATION AND E-ROOM FLOOR FRAMING PLANS	S101
SECOND & THIRD FLOOR FRAMING PLANS	S102
ROOF FRAMING PLANS	S103
STEEL COLUMN SCHEDULE AND STEEL BASE PLATE DETAILS	S201
TYPICAL FOUNDATION DETAILS	S301
TYPICAL SLAB ON GRADE DETAILS	S302
MISC. CONCRETE DETAILS	S303
FOUNDATION SECTIONS	S304
TYPICAL FRAMING DETAILS	S401
FRAMING SECTIONS	S402
TYPICAL WOOD FRAMING DETAILS	S501
TYPICAL WOOD FRAMING DETAILS	S502
TYPICAL CFS DETAILS	S601
TYPICAL MASONRY DETAILS	S701

IS INCLUDED	PHONE: 603.228.8880	FAX: 603.228.8881	WWW.SHEERR.COM
ENSURE THAT ALL SCOPE OF WORK		ר	<b>ARCHITECTURE</b>
N PURPOSES AND TO	48 Constitution Drive Bedford, NH 03110 Phone #: (603) 472-4488 Fax #: (603) 472-9747	www.tfmoran.com TFM Proj. # 95966.00	_
S. FOR COORDINATION		Sirugiura Enringers	
I'S AND SUB-CONTRACTORS RESPONSIBILITY TO REVIEW ALL DRAWINGS & SPECS. FOR COORDINATION PURPOSES AND TO ENSURE THAT ALL SCOPE OF WORK IS INCLUDED	Town of Henniker, New Hampshire Tucker Free Library Addition	31 Western Ave. Henniker, NH 03242	GENERAL STRUCTURAL NOTES
NTRACT	Job No. Scale	181	60
SUB-CO	Drawn By LPC3 / JW	Rev'd	By PES
S AND (	Date	XX/X	
NOTE: IT IS THE GC'S, CM		-	

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## CONCRETE (CONT.):

- 7. Cure and protect slabs for not less than seven (7) days with a curing compound conforming to ASTM C309 compatible with any intended floor overlay. Do not install finish flooring until the slab has adequately cured.
- 8. Elevated floor slabs shall be placed to the elevation indicated on the drawings. Provide maximum of 10% additional concrete as required due to formwork and floor framing deflection to achieve this top of slab elevation. The G.C. shall place concrete slabs with a [Class B or A, A-2,C] surface finish tolerance, as defined by ACI to allow for both framing deflection and a uniform slab thickness. Depressions in floors between high spots shall not be greater than  $\frac{5}{16}$ " below a 10'-long straight edge.
- Contractor shall coordinate additional ASTM Floor Flatness (FF) and Floor Levelness (FL) requirements with Owner and Owner's equipment.

10. Top of foundation walls shall be smooth and level.

- 11. Produce, place and protect concrete during periods of cold weather as outlined in ACI 306.1 "Standard Specification for Cold Weather Concreting" and during periods of hot weather as outlined in ACI 305.1 "Standard Specification for Hot Weather Concreting".
- 12. Horizontal construction joints shall be as indicated on the drawings. Vertical construction joints shall be approved by the architect and shall align with masonry wall control joints. Space vertical control or construction joints at concrete walls no more than 30'-0" on center. Construction joints shall be formed with a key, and reinforcing shall be lapped to develop the full tension capacity of the (smaller) bar.
- 13. Slab control joints, not shown on the drawings, shall be laid out in a square or rectangular fashion in accordance with ACI recommendations, Typical Slab-on-Grade Details and with the length not exceeding the width by 20%. No assurance is offered by TFM that random shrinkage cracking will not occur. Coordinate joint locations with Architect. Do not cut slab control joints on elevated composite slabs. Fill joints, unless otherwise noted with semi-rigid epoxy joint filler such as Metzger/McGuire Spal-Pro XL or equal.
- 14. Exposed concrete shall be rubbed immediately after removal of forms, see architectural drawings for finish type. Interior finish slab surfaces shall have a steel trowel finish unless noted on structural, architectural or civil drawings. All finishes of concrete surfaces shall be approved by the Owner/tenant, prior to construction.
- 15. Openings in concrete walls shall be located, sized, and reinforced (with the exception of small openings and/ or sleeves of a size that will not displace or interrupt the continuity of the reinforcing) as shown on respective details. Any alterations require approval of the Structural Engineer.
- 16. Column anchor bolts shall be set by template at column footings and piers.
- 17. No pipe shall pass through concrete without permission from the Structural Engineer. Pipe sleeves shall be provided and spaced a minimum of three (3) diameters apart. Pipes shall not pass through footings.
- 18. Unless otherwise noted, keys shall be 2"x4" with beveled sides.
- 19. Do not backfill foundation walls until the concrete has been in place for seven (7) days and attained 75% of its design compressive strength. Do not backfill retaining walls until floor framing and slab on grade has been placed. Foundations shall achieve the design strength prior to supporting structural steel.
- 20. [Concrete floor hardener shall be "Ashford Formula" manufactured by Curecrete Chemical Company or approved equivalent, coordinate locations w/ Owner].
- 21. Place epoxy/adhesive/acrylic anchors in materials at manufacturer recommended temperature ranges. If temperature ranges cannot be achieved, coordinate appropriate epoxy/adhesive/acrylic material substitute for approval with structural engineer.
- 22. Post installed anchors shall be installed in sound concrete / masonry in accordance with the manufacturer's recommendations / instructions. Reinforcing steel shall not be cut in order to install anchors.
- 23. Provide pockets in walls with steel bearing plates for support of all beams framing into/onto walls unless noted otherwise.
- 24. Notify Engineer of any wall and / or slab cracks of  $\frac{1}{8}$ " or greater for review.
- Submittals to the Structural Engineer are required for each concrete mix design to be used including mix designs, cementitious materials, aggregates, admixtures, and appropriate historic compressive strength test data per ACI 318. Hot and Cold Weather procedures and slab curing procedures. Epoxy/adhesive/acrylic/expansion anchor product data. Expansion joint materials, sealers and curing compound product data.

#### MASONRY:

- Masonry construction shall conform to applicable Building Code Requirements for Masonry 1. Structures (ACI 530 / ASCE 5 / TMS 402 ).
- 2. Concrete masonry units (CMU) shall be nominal thickness unless noted otherwise on the contract drawings.
- 3. Specified masonry compressive strength, f'm = 2,000 psi Inspected.
- Hollow load bearing concrete masonry units shall be in accordance with ASTM C90 4. specifications for Normal weight (average oven-dry density of 125 pcf or more and max. water absorption of 13 pcf) and Lightweight (average oven-dry density less than 105 pcf and max. water absorption of 18 pcf). Load bearing CMU shall have a minimum average net area compressive strength of 1,900 psi according to ASTM C140.
- 5. Hollow non-load bearing concrete masonry units shall be in accordance with ASTM C129. All non-load bearing partitions shall be light weight CMU.
- Mortar shall be ASTM C270 (property specification) Type S mortar with 28 day compressive 6. strength of 1,800 psi. Mortar shall be preblended (bag or bulk) dry masonry mortar mix containing Portland cement, hydrated lime and dried masonry sand. Mortar mix shall NOT contain mortar cement or masonry cement.
- 7. Grout shall be ASTM C476, fine grout with minimum 28 day compressive strength of 3,000 psi. Grout shall contain Portland Cement Type I/II.
- Vertical and horizontal deformed reinforcement shall be ASTM A615 grade 60. Horizontal joint reinforcement shall conform to ASTM A951, wire shall conform to A1064, mill galvanized coating according to ASTM A641 and hot-dipped galvanized at exterior walls according to ASTM ASTM A153
- 9. Prism tests according to ASTM E447 are required prior to work.
- 10. Grout CMU solid at expansion anchor locations.
- 11. Cores and bond beams with reinforcing shall be filled solidly with grout. Filling such cores and bond beams with mortar is strictly prohibited. In addition, care shall be exercised in keeping cores free from mortar droppings.
- 12. Minimum reinforcing requirements for reinforced CMU walls shall conform to the schedule shown on the contract drawings and the applicable building code requirements. Reinforcing shall be spliced as per the min. lap length shown within the masonry reinforcing schedule.
- 13. Grout shall be placed using low or high lift grouting procedures conforming to ACI/ASCE. Terminate grout pours 1-1/2 inches below top course of placement. Provide clean-outs and transparent window covers at bottom of all high lift grouted cores.

#### MASONRY (CONT.):

14. Vertical reinforcing shall be securely held in proper alignment and position during grouting operations by using "Rebar Positioners," as manufactured by Wire Bond or approved equal. The product, in addition, shall allow for guiding the spliced reinforcing dropped from the top of the lift.

- Masonry shall be braced during construction. Brace spacing shall not exceed ten times the wall 15. thickness but not less than the procedures listed under NCMA-TEK 72. Design of all bracing is to be provided by the Contractor.
- 16. Provide full height vertical reinforcement at each side of control joints, window, door, and wall openings, at all ends of walls and corners. Reinforcing shall be grouted solid and match the diameter of the typical wall reinforcing, unless noted.
- 17. Provide pockets in walls with steel bearing plates for support of all beams framing into/onto walls unless noted otherwise.
- 18. Submittals to the Structural Engineer are required for mortar and grout mix designs with test results, masonry unit test data, accessories, hot and cold weather procedures and grouting method

#### **REINFORCING STEEL**

- Reinforcing steel shall be deformed bars, free from loose rust and scale, and conforming to ASTM A615, Grade 60.
- Welded wire fabric (WWF) shall conform to ASTM A185. Lap 1.5 squares at joints and tie at 3'-0" 2 o.c. Furnish WWF in flat sheets.
- Welded wire fabric (WWF) at slabs on grade shall be supported on chairs or bolsters spaced at 3. 24"-36" or less, as required to maintain WWF at indicated clear cover location.

Clear concrete cover over bars shall be as conditions not noted):	s follows unless otherwise noted (see ACI 318 for
Footings:	3 inches (bottom), 2 inches (top and side)
Walls and Piers (exposed to earth):	2 inches (side)
Walls and Piers (interior):	1 1/2 inches (side)
Elevated slabs:	1 inch (top)
Slab on grade:	2 Inches (top)

- Accessories in contact with forms to be removed shall have upturned legs and be plastic-dipped after fabrication. Accessories for reinforcing shall be in accordance with ACI current edition.
- 6. Lap reinforcing to develop the full tension capacity of the (smaller) bar. Provide Class B splice unless noted otherwise.
- 7. No bars shall be cut or omitted in the field because of sleeves, duct openings or recesses. Bars may be moved aside without change in level with the prior approval of the Structural Engineer.
- 8. Shop or Erection drawings shall be submitted to the Structural Engineer showing the layout, spacing, lap lengths, quantity and sizes of all concrete and masonry reinforcing.
- Submittals to the Structural Engineer are required for product data of all accessories, including 9. WWF, chairs, bolsters and mechanical connectors.

#### STRUCTURAL STEEL

- Fabricate and erect structural steel in accordance with the applicable "Specification for Structural Steel for Buildings" and the "Code of Standard Practice" of AISC. Welding shall conform to the requirements of the "Structural Welding Code" of the American Welding Society.
- Structural steel wide flange shapes shall conform to ASTM A992 (Fy = 50 ksi). Hollow Structural 2. Sections (HSS) shall conform to ASTM A500, Grade B (Rectangular Fy = 46 ksi, Round Fy = 42 ksi). Pipe shall be ASTM A53, Grade B (Fy = 35 ksi). Structural steel channels, misc. shapes, plates, and angles shall conform to ASTM A36 (Fy = 36 ksi), unless otherwise noted.
- 3. Do not splice structural steel members without written approval of the Structural Engineer.
- Bolted connections shall be made with three-quarter inch diameter high strength, ASTM A325-N bolts, unless otherwise noted. Connections at moment frames, braced frames, column splices and hangers shall be made with three-quarters inch diameter A325-SC (Slip critical) bolts, unless otherwise noted.
- All beam to beam and beam to column connections shall be double angle connections
- 6. Shop connections, unless otherwise noted, shall be welded. Unless otherwise noted, beam connections shall provide shear capacity to support a reaction R equal to half the total uniform load capacity of the beam for given shape, span and steel specification (AISC) taking account for the effect of concentrated loads.
- 7. Shop connections, unless otherwise noted, shall be welded. Unless otherwise noted, beam connections shall provide shear (kips)/ moment (kip\*ft) capacity to support an ASD reaction equal to that which is shown at the end of the beam on the framing plans.
- Shop connections, unless otherwise noted, shall be welded. Unless otherwise noted, beam connections shall provide shear/ moment capacity to support an ASD reaction equal to the following: W16 or greater:

18 kips, +/- 30 kip\*ft (at moment frames only) W14 or smaller: 10 kips

9. The following factors shall be applied to the composite beam end reactions determined from the uniform load capacity of the beam (noted above), for the design of the shear connections: F=1.3 W36 W30

1 1.0.	1100, 1100
F=1.5:	W27, W24, W21
F=1.7:	W18, W16, W14
F=2.0:	W12, W10, W8

- 10. Shop camber steel beams as shown on the drawings. Camber tolerance shall be -0" or +1/2". Camber shall be measured with beam web vertical (erected condition) under its own dead load.
- 11. Anchor bolts shall be ASTM F1554 Gr. 36 headed bolts of the diameters and dimensions detailed, unless otherwise noted on the drawings. High strength (HS) anchor bolts shall be ASTM F1554 Gr. 55, Supplement 1 (weldable) headed bolts. Anchor bolts shall be set by template. Hooked ("J" type) anchor bolts are not permitted.
- 12. Welding electrodes shall conform to AWS A5.1 E70XX series with proper rod to produce optimum weld (low hydrogen).
- 13. Unless otherwise noted, bolted connections with slotted holes shall be field-welded with one-quarter inch fillet welds after final field adjustment.
- 14. Provide 3/8" minimum fitted stiffener plates each side at beams' web framing over columns and at beams supporting columns above.
- 15. Provide 1/4" thick steel leveling plate on 3/4" min. non-shrink grout under all column base plates unless otherwise noted. Leveling plates shall be set and grouted prior to erecting columns.
- 16. Provide all angles, plates, anchors, bolts, etc., shown on architectural drawings.
- 17. Lintels for exterior masonry and structural steel exposed to weather shall be hot-dip galvanized according to ASTM A123.
- 18. Provide L4x4x1/4 minimum steel deck support angles as required at columns where structural members do not frame in at all four sides, at changes in deck span direction, at changes in floor and/ or roof planes over structural members, along diagonal cuts, and at all openings. Provide additional deck support where any framing connection prevents the deck from being adequately supported by framing members.
- 19. Provide holes in beam top flanges for nailer attachment at steel supporting wood framing, see schedules, details and notes for size and spacing.

20. Steel Primer:

General Primer: Standard Alkyd Primer applied at 2.5 - 3.5 mil DFT shall be used as the standard of quality and performance. Color: grey

[Primer for steel with fireproofing: Tnemec Series 394 Perime-Prime (or approved equal) applied at 2.5 - 3.5 mils DFT shall be used as the standard of quality and performance.] [Do not prime steel with fireproofing, see Architect for extent of areas to receive spray on fireproofing.]

Touch up paint in the field by the Contractor, unless otherwise noted.

- 21. The Steel fabricator is responsible for the design and detailing of all connections including moment connections, braced frame connections and beam and/or column stiffeners and doubler if required. All connections must be designed by a registered professional engineer in the state i which the project is being constructed. Certification of this design shall be provided with the shop drawings submittal for review by the engineer of record.
- 22. The design of steel stairs, hand rails, and guard rails are delegated for design by others and of the general contractor. All must be designed by a registered professional engineer in the state in which the project is being constructed. Certification of this design shall be provided with the shop drawings submittal for review by the engineer of record.
- 23. Shop or Erection drawings shall be submitted to the Structural Engineer for fabrication and erection of structural steel and stairs, prior to fabrication.
- 24. Submittals to the Structural Engineer are required for mill tests and sealed connection calculations and sealed stair calculations.

#### COLD-FORMED STEEL FRAMING:

- 1. Design, Fabricate, and Install cold-formed steel framing members and accessories plumb, square, and true to line, and with connections securely fastened, according to manufacturer's written recommendations and requirements, the requirements of the applicable International Building Code (IBC) and the applicable edition of the American Iron and Steel Institute (AISI) Specification.
- 2. Fasten cold-formed steel framing members by welding or screw fastening, as standard with fabricator. Wire tying of members is not permitted.
- Comply with AWS D1.3 requirements and procedures for welding, appearance and quality of welds, and methods used in correcting welding work.
- 4. Locate mechanical fasteners and install according to shop drawings, with screws penetrating joined members by not less than three exposed screw threads.
- 5. Provide cold-formed roof, floor, and wall metal framing members, connectors, and fasteners in accordance with the plan and detail drawings.
- 6. Roof, floor, and wall framing members shall be standard C-shaped steel joists, unpunched, with stiffened flanges, complying with ASTM C 955 and as follows:
- 7. Provide cold-formed steel framing capable of withstanding design loads within limits and under conditions indicated.

Design loads shall be in accordance with applicable IBC Sections of Wind Loads and Earthquake Loads for components and cladding.

Design Exterior Non-load-bearing wall framing for a maximum horizontal deflection of 1/360 of the wall span.

Minimum Uncoated Thickness: 20 GA.

Standard C-shaped steel studs with a Minimum Flange Width: 1 5/8"

Specified Yield Strength: Fy = 33 ksi (18 GA and thinner) Fy = 50 ksi (16 GA and thicker) Maximum studs spacing of 16" on center.

- 8. All galvanized studs, track, bridging, and accessories shall be formed from steel having a galvanized coating meeting the requirements of ASTM A 653.
- Isolate non-load-bearing steel framing from building structure to prevent transfer of vertical loads 9. while providing lateral support. Connect vertical deflection clips bypassing studs and anchor to primary building structure
- 10. Install horizontal bridging in walls studs, spaced in rows indicated on Shop Drawings but not more than 48 inches apart. Fasten at each stud intersection. Bridging shall be determined by stud manufacturer.
- 11. All framing members shall be manufactured and supplied by Dietrich Industries, MarinoWare or approved equal.
- 12. Shop / Erection drawings shall be submitted to the Structural Engineer showing the layout, spacing, sizes, thickness, and types of cold formed steel framing, fastening and anchorage details, including mechanical fasteners.
- 13. Submittals to the Structural Engineer shall include structural analysis data signed and sealed by the professional engineer, registered in the state of the project's construction, responsible for their preparation.

	W	DOD:						
	1.	Work shall be in accordance with the applicable American Wood Council, ANSI / AF&PA, "National Design Specification for Wood Construction (NDS)" including "Design Values for Wood Construction", National Forest Products Association.						
	2.	New wood for structural use shall have a moisture content as specified in the "National Design Specification for Wood Construction."						
	3.	Wood construction shall conform to applicable IBC, Chapter and Section for "Conventional Light-frame Construction."		_				
rs	4.	Sheathing panels shall be marked with the American Plywood Association (APA) trademark and shall meet the latest U.S. Product Standard PS 1 or APA PRP-108 Performance Standards.					· —	
n	5.	All wall sheathing panels shall be 1/2" thick, 32/16 (minimum), APA Rated and all sheathing panel edges shall be blocked, unless otherwise noted. Fasten with 8d common nails spaced at 6"o.c. at panel perimeter supported edges and 12"o.c. at interior intermediate supports (field) with 1 3/8" min. fastener penetration, unless otherwise noted. Lay wall sheathing with long dimension perpendicular to support members.				3.228.888	3.228.888	COM.
μ	6.	All floor sheathing panels (sub-floor) shall be 3/4" thick, APA Rated Sturd-I-Floor, 48/24 (minimum), T&G, Exposure 1 meeting DOC PS1 or PS2. Sheathing to be glued with adhesives meeting APA Spec. AFG-01 and fastened with 8d common nails spaced at 6"o.c. at panel perimeter supported edges and 12"o.c. at interior intermediate supports (field) with 1 3/8" min. fastener penetration, unless otherwise noted. Lay floor sheathing with long dimension perpendicular to support members and stagger sheathing panels in a row one half panel length with previous row.				PHONE: 603		WWW.SHEERR
	7.	All roof sheathing panels shall be 5/8" thick <u>[3/4" at flat roofs]</u> , 40/20 (minimum), C-D Exterior grade, APA rated Exposure 1 meeting DOC PS1 or PS2. Fasten with 8d common nails spaced at 6"o.c. at panel perimeter supported edges and 6"o.c. at interior intermediate supports (field) with 1 3/8" min. fastener penetration, unless otherwise noted. Lay roof sheathing with long dimension perpendicular to support members and stagger sheathing panels in a row one half panel length with previous row. Support edges of sheathing at roof pitch breaks with blocking.			5		2	
	8.	OSB OPTIONs???				10		I H
	9.	Framing for walls, joists, rafters beams and headers shall be Spruce-Pine-Fir (SPF) No. 1/ No. 2, unless noted. Dimensioned lumber represents nominal sizes. See minimum properties below:						E
	10.	Wood exposed to the weather or in contact with concrete or masonry shall be pressure treated (P.T.) Southern Pine No. 1, unless noted. See minimum properties below:						
	11.	Laminated Veneer Lumber (LVL) members shall be 1.9E Trus Joist Microllam LVL as manufactured by Weyerhaeuser or approved equivalent. See minimum properties below:				VI		RCH
	12.	Laminated Strand Lumber (LSL) joists and stud members shall be 1.3E Trus Joist TimberStrand LSL as manufactured by Weyerhaeuser or approved equivalent. See minimum properties below .				2,		∎≖
	13.	Laminated Strand Lumber (LSL) beam and header members shall be 1.55E Trus Joist TimberStrand LSL as manufactured by Weyerhaeuser or approved equivalent. See minimum properties below.			2	ive 0 2-4488	3747 300	
)	14.	Wood framing shall have the minimum design values:Min. Design ValuesSpecies / MaterialE (psi)Fb (psi)Fv (psi)Spruce-Pine Fir (SPF) No. 1/ No. 2:1.4e6 $875$ 135Southern Pine (SP) No. 1/1.9E1.4e61,100175Laminated Veneer Lumber (LVL) 1.9E members:1.9e62,600285Parallel Strand Lumber (PSL) 2.0E (Beams):2.0e62,900290Parallel Strand Lumber (PSL) 1.8E (Columns):1.8e62,400190Laminated Strand Lumber (LSL) 1.3E (Joists / Studs):1.3e61,700400Laminated Strand Lumber (LSL) 1.55E (Beams/ Headers):1.55e62,325310Glulam 24F-V4.E (Beams):1.9e62,400265				48 Constitution DI Bedford, NH 0311 Phone #: (603) 47	<ul> <li>Fax #: (603) 472-9747</li> <li>www.tfmoran.com</li> <li>TFM Proi. # 95966.00</li> </ul>	
	15.	Pressure treated (P.T.) wood shall meet the following standards for each condition of use:         Pressure       Min.       AWPA         Condition       Treatment       Retention       Category         Interior Construction:       (Wood not exposed to weather, in contact with concrete or masonry)       CCA, ACQ-C,D       .25       UC2         MCA-C       .05       UC2			5		tructural	EIIGIIIGELS
S		Above Ground, exterior construction:         (Beams, joists and stringers       CCA, ACQ-C,D       .25       UC3B         not in contact with the ground)       CA-B       .1       UC3B         MCA-C       .05       UC3B					5	
		Ground Contact, fresh water:       ImcA-C       .05       0C3B         Ground Contact, fresh water:       (Posts and members exposed to weather and in ground contact)       CCA, ACQ-C,D       .4       UC4A         MCA-C       .21       UC4A         MCA-C       .15       UC4A		30	ひ ざ	nire		
		Treated Sheathing				npsł	42	NOTES
		Chromated Copper Arsenate (CCA), Alkaline Copper Quaternary (ACQ), Copper Azole (CA) and Micronized Copper Azole (MCA)	SET			Han dditic	H 032	
/	16.	Field treat cut ends of P.T. wood with Copper Naphthenate preservative such as Copper-Greene. Wood to steel and wood to wood bolted connectors shall be made with ASTM A307 bolts with flat washers. Bolt holes in wood shall be 1/32" larger than the bolt. Wood nailers shall be fastened with (2) rows of 1/2" diameter bolts staggered at 2'-0" o.c. unless otherwise noted.	BUDGET S	NOT NOT		er, New Hampshire	31 Western Ave. Henniker, NH 03242	STRUCTURAL
	17.	Fastening Schedule:Plate to Stud, Direct2-16dStud to Plate, Toenail4-8d	BUI	/2020		Town of Henniker Tucker Free L	stern Ave	
		NOTE: SEE APPLICABLE IBC TABLE "FASTENING SCHEDULE" FOR FASTENING/ NAILING REQUIREMENTS NOT SHOWN.		DATE: 1/17/2020		of H Tuck	1 We	GENERAL
	18.	The lateral bracing system includes plywood wall and roof sheathing. Contractor shall provide temporary bracing as required to laterally support the structure during construction.		DATE: 1/17/202		UM No	6	CH GH
	19.	Provide lateral support at all bearing points and along compression edges at intervals of 24"o.c. or closer.				Ч		
	20.	Minimum section width = $1 3/4$ ". The $3 1/2$ ", $5 1/4$ ", and 7" members may be combinations of $1 3/4$ " members. Follow manufacturers guidelines for Multiple Member Connections for side loaded beams.				Job No.	10,	160
	21.	Wood Construction Connectors shall be manufactured by Simpson Strong-Tie Co., MiTek Industries, Inc. or approved equal and installed in accordance with the manufacturers recommendations.				Scale	10	160
	22.	All wood fasteners and hangers in contact with pressure treated (P.T.) and or fire retardant treated (FRT) lumber are to be stainless steel or hot dipped galvanized (min 2.0 oz/ft^2). Hangers located in Ocean/Water Front environments shall be stainless steel.				Drawn By Designer Date	Rev'o	d By necker
	23.	All non-load bearing / non-structural walls shall be held down from joists / rafters / trusses above.			ξ		01/	/16/20
	24.	The Contractor shall retain the service of a certified lumber grader to identify the existing wood members that have rot, insect damage, severely cracked, checked, split or otherwise have deteriorated to the point that it needs to be either reinforced or replaced. Existing conditions to be identified before proceeding with any work.			σ́ _	Revised		
	25.	Shop or Erection (placement) drawings shall be submitted to the Structural Engineer showing the layout, sizes and anchorage details for all engineered lumber framing.						
	26.	Submittals to the Structural Engineer shall include engineered lumber and hanger / connections product data, engineered lumber product data		1 1 1 1 1 1 1	2  -			

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FOR BUDGET ONLY

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## PRE-E

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- 2. The m plant. trusse
- 3. Trusse coordi

Top Chord Wind Load       (see the "DESIGN LOADS", section of the general notes)       Figure 10 ps/standard standard standar	RE-ENGINEERED WO	DOD TRUSSES:	<u>S1</u>	RUCTURAL TESTS AND INSPECTIONS:				
<text><text><text><text><text><text><text><text><text><text><text><text><text><text><text><text><list-item><list-item><text><list-item><text><list-item><text><list-item><text><list-item><text><list-item><text></text></list-item></text></list-item></text></list-item></text></list-item></text></list-item></text></list-item></list-item></text></text></text></text></text></text></text></text></text></text></text></text></text></text></text></text>			1.	masonry construction, steel construction and other applicable construction shall be promptly				
<text><text><text><text><text><text><text><text><text></text></text></text></text></text></text></text></text></text>	plant. Proof of certification shall b		2.	Tests and Inspections shall be completed in accordance with the applicable IBC Special Inspection chapter. Refer to and coordinate with the Statement of Special Inspections/Quality				
<text><text><text><text><text><text><text><text><text><text><text><text><text><text><text><text><text><text><text><text></text></text></text></text></text></text></text></text></text></text></text></text></text></text></text></text></text></text></text></text>			З	inspections for each building material/system.				
<form><text><text><text><text><text><text><text><text><text><text><text><text><text></text></text></text></text></text></text></text></text></text></text></text></text></text></form>		in order to calculate the horizontal projection)	3.	state the project is located in. Unless specifically stated in writing and listed on the Statement of Special Inspections, TFM is not the Special Inspector or Special Inspections Coordinator and this		00	$\boldsymbol{\infty}$	
<text><text><text><text><text><text><text><text><text><text><text><text><text><text><text><text><text></text></text></text></text></text></text></text></text></text></text></text></text></text></text></text></text></text>	Top Chord Snow Load	20 psf (min.) (see the "DESIGN LOADS" section of the general notes)	4.	provided to TFM at the completion of the project. The document shall be stamped by a		.228.	.228.	NUJ
<text><text><text><text><text><text><text><text><text><text><text><text><text><text><text><text><text><text><list-item><list-item><text><text><list-item><list-item><text><text><list-item><list-item><text><text><list-item><text><list-item><text><text></text></text></list-item></text></list-item></text></text></list-item></list-item></text></text></list-item></list-item></text></text></list-item></list-item></text></text></text></text></text></text></text></text></text></text></text></text></text></text></text></text></text></text>	Bottom Chord Dead Load	section of the general notes) 10 psf 10 psf min. (see the "DESIGN LOADS"	5.	requirements. Additional testing and inspecting, at Contractor's expense, will be performed to	DED	E: 60	0	
<text><text><text><text><text><text><text><text><text><text><text><text><text><text><text><text><text><text><list-item><list-item><text><text><list-item><list-item><text><text><list-item><list-item><text><text><list-item><text><list-item><text><text></text></text></list-item></text></list-item></text></text></list-item></list-item></text></text></list-item></list-item></text></text></list-item></list-item></text></text></text></text></text></text></text></text></text></text></text></text></text></text></text></text></text></text>	-	As indicated on plans	6.		NCLU	NOH	FAX:	mmm
<text><text><text><text><text><text><text><text><text><text><text><text><text><text><text></text></text></text></text></text></text></text></text></text></text></text></text></text></text></text>	snow loading, or valley sno wind loading as specified ir	w conditions, as well as loads and diagrams shown and any the project building code (see the DESIGN LOADS / CODE		<ul><li>6.2. Test each mortar type in conformance with ASTM C780.</li><li>6.3. Test grout in each type of wall construction in conformance with ASTM C1019 using inch</li></ul>	<u></u>	1		
<text><text><text><text><list-item><list-item><list-item><list-item><list-item><list-item><list-item><list-item><list-item><list-item><list-item><list-item><list-item><list-item><list-item><list-item><list-item><list-item><list-item><list-item><list-item><list-item><list-item><list-item><list-item></list-item></list-item></list-item></list-item></list-item></list-item></list-item></list-item></list-item></list-item></list-item></list-item></list-item></list-item></list-item></list-item></list-item></list-item></list-item></list-item></list-item></list-item></list-item></list-item></list-item></text></text></text></text>	Design all trusses for the following non-bearing partitions:			<ul><li>6.4. Test concrete masonry units (CMU), for each type required, per ASTM C140.</li><li>6.5. Sample and test masonry prisms in conformance with ASTM C1314.</li><li>6.6. Inspect fully grouted, reinforced masonry cores.</li></ul>	OF WO	10		
<text><text><text><text><text><text><text><text><text><text><text><text><list-item><list-item><text><text><text><list-item><text><text><list-item><text><text></text></text></list-item></text></text></list-item></text></text></text></list-item></list-item></text></text></text></text></text></text></text></text></text></text></text></text>	Live/ Snow/ Wind Load:			6.8. Submit test results to the Architect of Record and Structural Engineer within 7 days. Report				
<text><text><text><text><text><list-item><list-item><list-item><list-item><list-item><list-item><list-item><list-item><list-item><list-item><list-item><list-item><list-item><list-item><list-item><list-item><list-item><list-item><list-item><list-item><list-item><list-item><list-item><list-item><list-item><list-item><list-item><list-item></list-item></list-item></list-item></list-item></list-item></list-item></list-item></list-item></list-item></list-item></list-item></list-item></list-item></list-item></list-item></list-item></list-item></list-item></list-item></list-item></list-item></list-item></list-item></list-item></list-item></list-item></list-item></list-item></text></text></text></text></text>	while providing lateral support.		7.	C 172 shall be performed according [to the Statement of Special Inspections:] to the following	SC			
<text><text><text><text><text><text><text><text><text><text></text></text></text></text></text></text></text></text></text></text>	until delegated design is complete	e. TFM bracing details and locations are intended to show		mixture exceeding 5 cu. yd., but less than 50 cu. yd., plus one set for each additional 50 cu. yd. or fraction thereof. When frequency of testing will provide fewer than five	THAT /			
<text></text>				compressive-strength tests for each concrete mixture, testing shall be conducted from at least five randomly selected batches or from each batch if fewer than five are used.		\U		
<text></text>	applied continuous lateral restrain Minimum member properties fl Minimum web member size - 2	t (CLR). Design is subject to the following conditions: =875 psi, e=1400 ksi x4		<ul> <li>but not less than one test for each day's pour of each concrete mixture. Perform additional tests when concrete consistency appears to change.</li> <li>7.3. Air Content: ASTM C 231, pressure method, for normal-weight concrete; one test for each composite sample, but not less than one test for each day's pour of each concrete mixture.</li> </ul>	2			
<text><text><text><text><text><text><text><text><text></text></text></text></text></text></text></text></text></text>	Minimum brace (CLR) size - 2 Web lengths less than 6'-0" are	v4 e not permitted to receive (CLR)		and below and when 80 deg F and above, and one test for each composite sample. 7.5. Compression Test Specimens: ASTM C 31.	ω I	n Drive )3110 ) 472-4	17'2-974 com 5966.00	2200.0
<text><text><text><text><text><text><text><text><text><text><text><text><text></text></text></text></text></text></text></text></text></text></text></text></text></text>	R) locations shall be clearly show Provide a "T" or "L" brace whe	n on truss design drawings (TDD)		sample.	OSE	stitutio d, NH C #: (603	ლ ლ #	ŧ
<ul> <li>No. The a number latter increase on the meter properties or provide where the appropriate provide or provide or provide where the appropriate provide where the approprise provide provide provide</li></ul>	The truss design drawings (TD	D) shall show "T" or "L" brace size, length, location and		7.8. Compressive-Strength Tests: ASTM C 39; test one laboratory-cured specimens at 7 days and one set of two specimens at 28 days and one laboratory-cured specimen at 56 days if	PURP	48 Con Bedforc Phone	Fax #: ( www.tfi TFM Pi	
<text><text><text><text><text><text><text><list-item><list-item><text><list-item></list-item></text></list-item></list-item></text></text></text></text></text></text></text>	applied reinforcement in lieu o	f (CLR), "T" or "L" braces.		7.9. Test results shall be reported in writing to Architect and Engineer, concrete manufacturer, and Contractor within 48 hours of testing. Reports of compressive-strength tests shall contain Project identification name and number, date of concrete placement, location of	VATION		œ	_
<text><text><text><text><text><text><text><list-item><list-item></list-item></list-item></text></text></text></text></text></text></text>	lateral restraint is required.			Work, design compressive strength at 28 days, concrete mixture proportions and materials, compressive breaking strength, and type of break for both 7- and 28-day tests.	RDI	2	P	
The contract in the manupping the function at all activity the maximum of the machine with the manupping the function of all welts, altakes, mease, contract, contracts on the maximum of the machine with the manupping the function.	connections necessary for the ins	tallation of the trusses.		when test results indicate that slump, air entrainment, compressive strengths, or other requirements have not been met, as directed by Structural Engineer. Additional testing			uetur	neers
<text><text><text><text><text><text><text><text><text><text></text></text></text></text></text></text></text></text></text></text>	galvanized fasteners and connect	ors at all FRT trusses.		replaced or additional work with specified requirements. 7.11. Deficiencies: Correct deficiencies in the Work that test reports and inspections indicate			Str	
<ul> <li>Item table table table table table tables that the table tables ta</li></ul>	The contractor shall ensure prope	r handling, bracing, and lateral restraint is in accordance with	8.	Wood Construction Inspections: Inspection of the wood construction shall be performed	SPE			T
drawings See details and approved truss ship drawings for final humine ellip information.       All temporary braining design and erection procedures are the responsibility of the General       about the approved human ship drawings for final humine ellip information.         All temporary braining design and erection procedures are the responsibility of the General       about the approved human ship drawings for final humines ellip information.       about the approved human ship drawings for final humines ellip information.       about the approved human ship drawings for final humines ellip information.       about the approved human ship drawings for final humines ellip information.       about the approved human ship drawings for final humines ellip information.       about the approved human ship drawings for final humines ellip information.       about the approved human ship drawings for final humines ellip information.       about the approved human ship drawings for final humines ellip information.       about the approved human ship drawings for final humines.       about the approved human ship drawings for final humines.       about the approved human ship drawings for final humines.       about the approved human ship drawings for final humines.       about the approved human ship drawings for final humines.       about the approved human ship drawings for final humines.       about the approved human ship drawings for final humines.       about the approved human ship drawings for final humines.       about the approved human ship drawings for final humines.       about the approved human ship drawings for final humines.       about the approved human ship drawings for final humines.       about the approved human ship drawings for final humines		ation of all vents, stacks, risers, drains, etc. before trusses are				e		
Contractor / Erector.  Temporary installation retainer threading and the permanent individual trues member retainer threading of all tures with spans over dDF alls be designed, stamped and submitted by a learned production state in the state where the project is booking and interactors and interactors and with a contractor Dormetia. Verify and weeks and verify compliance with the Contract Dormetia besides and verify compliance with the Contract Dorve weeks in perform threading and prove and perform threading and and brain and a contract Director to inspect the inspect the inspect for weeks and perform threading and anchorage details, all brain greating indicated on threading and anchorage details, all brain greating indicated on threading and prove applications of plant to conditional to the Structural Desenvolution and interactor of their responsibility to thei	All roof trusses shall have mecha installed at each bearing location bearing plate shall be capable of	and end of each truss. Mechanical connection of truss to withstanding uplift loads shown on approved truss shop			DRAWIN	ampshi lition	33242	
<ul> <li>The started type ingree qualifies to perform the work in the stale where the project is context.</li> <li>All foor and parallel chord trusses with ages are work in the stale where the project is context.</li> <li>Structural stell inspection is and verify compliance with the Contract Documents and verify compliance with the Contract Documents.</li> <li>Structural Document) drawings and there to perform the work in the stale with the Contract Documents and verify compl</li></ul>	All temporary bracing design and			Wood Framing: [Periodically][Continuously] inspect wood structural panel sheathing size, configuration, blocking, and fastening (including spacing, diameter, and length of nails or		ew H ry Adc	(er, NH	
<ul> <li>a) located</li> <li>b) a located professional engineer qualified to perform the work in the state where the project is located.</li> <li>c) (3) (3) (3) (3) (3) (3) (3) (3) (3) (3</li></ul>	restraint/bracing for all trusses wit	h spans over 60' shall be designed, stamped and submitted by		thickness. [Periodically][Continuously] inspect wood framed shear wall end posts, holdowns, straps, and other components and verify compliance with the Contract Documents.	\$EVIE	0	Hennil	
be in accordance with applicable section of IBC. Truss shop drawings shall be designed. stamped and submitted by a licensed professional engineer qualified to perform the work in the state where the project is located. Submittal shall include all loading combinations, a full report for each truss, including but not limited to: pitch, span camber, configuration deflection and spacing for each trues structural required specifies, sizes and stress grades of lumber; splice details; connector plate size, material, finish, design values, orientation and location; member forces, reactions, baracing requirements; temporary/ permanent lateral truss restraint layouts and defails. Shop or Erection (placement) drawings shall be submitted to the Structural Engineer showing the truss layout and spacing, fastening and anchorage details; all bracing requirements and all truss to truss torus shoets, bracing indicated on TFM drawings and other necessary bracing shall be coordinated by the supplier and shown on a layout drawing that shall be submitted to the Engineer for review and approval to verify compliance with Engineer's design intent of permanent bracing. Submittals to the Structural Engineer shall include Truss Design Drawings, truss layout drawings and truss bracing layout drawings, signed and sealed by the professional engineer, registered in the state of the project's construction, responsible for their preparation. Submittals to the Structural Engineer shall include Truss Design Drawings, truss layout drawings and truss bracing layout drawings, signed and sealed by the professional engineer, registered in the state of the project's construction, responsible for their preparation. Submittals to the Structural Engineer shall include Truss Design Drawings, truss layout drawings and truss bracing layout drawings, signed and sealed by the professional engineer, registered in the state of the project's construction, responsible for their preparation. Submittals to the Structural Engineer shall include		qualified to perform the work in the state where the project is	9.	Structural steel inspections:	2	nike ree	Ave.	
be in accordance with applicable section of BC. Truss shop drawings shall be designed. stamped and submitted by a licensed professional engineer qualified to perform the work in the state where the project is located. Submittal shall include all loading combinations, a full report for each truss, including but not limited to: pitch, span camber, configuration deflection and spacing for each truss required spices, sizes and truss escriptical details; connector plate size, material, finish, design values, orientation and location; member forces, reactions, bearing requirements; temporary permanent lateral truss orientation and location; member forces, reactions, bearing requirements; temporary permanent lateral truss escriptical details. Shop or Erection (placement) drawings shall be submitted to the Structural Engineer showing truss to truss connectors. Truss bracing indicated on TFM drawings and other necessary bracing shall be coordinated by the supplier and shown on a layout drawing that shall be submitted to the Engineer for review and approval to verify compliance with Engineer's design intent of permanent bracing. Submittals to the Structural Engineer shall include Truss Design Drawings, truss layout drawings and truss bracing jayout drawings, signed and sealed by the professional engineer, registered in the state of the project's construction, responsible for their preparation. Submittals to the Structural Engineer shall include Truss Design Drawings, truss layout drawings and truss bracing jayout drawings, signed and sealed by the professional engineer, registered in the state of the project's construction, responsible for their preparation. Submittals to the Structural Engineer shall include Truss Design Drawings, truss layout drawings and truss bracing jayout drawings, signed and sealed by the professional engineer, registered in the state of the project's construction, responsible for their preparation. Submittals to the Structural Engineer shall include Truss Design Drawings tr	(max.), do not use 2x4 bracing. S	trong back bracing shall be attached to each truss w/ (3) 10d		<ul> <li>9.1. AWS certified inspector to Inspect both shop and field welds and verify compliance with the approved shop drawings and Contract Documents as follows:</li> <li>9.1.1. Complete and partial joint penetration groove welds: Inspect and perform ultrasonic</li> </ul>		<sup>-</sup> Heni cker F	Vestern ,	
Shop or Erection (placement) drawings shall be submitted to the Structural Engineer showing the truss layout and spacing, fastening and anchorage details, all bracing requirements and all truss to truss connectors. Truss bracing indicated on TFM drawings and other necessary bracing shall be coordinated by the supplier and shown on a layout drawing that shall be submitted to the Engineer for review and approval to verify compliance with Engineer's design intent of permanent bracing. Submittals to the Structural Engineer shall include Truss Design Drawings, truss layout drawings and truss bracing layout drawings, signed and sealed by the professional engineer, registered in the state of the project's construction, responsible for their preparation.	be in accordance with applicable stamped and submitted by a licen state where the project is located. for each truss, including but not li spacing for each type of truss req	section of IBC. Truss shop drawings shall be designed, sed professional engineer qualified to perform the work in the Submittal shall include all loading combinations, a full report mited to: pitch, span camber, configuration deflection and uired; species, sizes and stress grades of lumber; splice details;		<ul> <li>9.1.2. Multi-pass fillet welds, single-pass fillet welds larger than 5/16", and plug and slot welds: Visually inspect 100% of welds and perform magnetic particle tests as required by inspector if defects are observed from visual inspection.</li> <li>9.1.3. Single-pass fillet welds smaller than 5/16": Visually inspect 50% of welds and perform magnetic particle tests as required by inspector if defects are observed from visual inspection.</li> </ul>		Town of Tu	31 \	
recessary bracing shall be coordinated by the supplier and shown on a layout drawing that shall be submitted to the Engineer for review and approval to verify compliance with Engineer's design intent of permanent bracing. Submittals to the Structural Engineer shall include Truss Design Drawings, truss layout drawings and truss bracing layout drawings, signed and sealed by the professional engineer, registered in the state of the project's construction, responsible for their preparation.	reactions, bearing requirements; t Shop or Erection (placement) dra	emporary/ permanent lateral truss restraint layouts and details. wings shall be submitted to the Structural Engineer showing		locations, and connection details such as bracing, stiffening, and proper application of joint details and verify compliance with the approved shop drawings and Contract Documents.	ACTOR	Job No.		_ ^
necessary bracing shall be coordinated by the supplier and shown on a layout drawing that shall be submitted to the Engineer for review and approval to verify compliance with Engineer's design intent of permanent bracing. Submittals to the Structural Engineer shall include Truss Design Drawings, truss layout drawings and truss bracing layout drawings, signed and sealed by the professional engineer, registered in the state of the project's construction, responsible for their preparation.	truss to truss connectors.		10.	Notify engineer of progress of construction for coordination of site observations per Chapter 17 of the International Building Code (IBC). These observations are intended for review of		Scale	18′	1
Submittals to the Structural Engineer shall include Truss Design Drawings, truss layout drawings and truss bracing layout drawings, signed and sealed by the professional engineer, registered in the state of the project's construction, responsible for their preparation.	necessary bracing shall be coordi be submitted to the Engineer for r	nated by the supplier and shown on a layout drawing that shall			SUB-	-		
	and truss bracing layout drawings	, signed and sealed by the professional engineer, registered in			AN		01	/1
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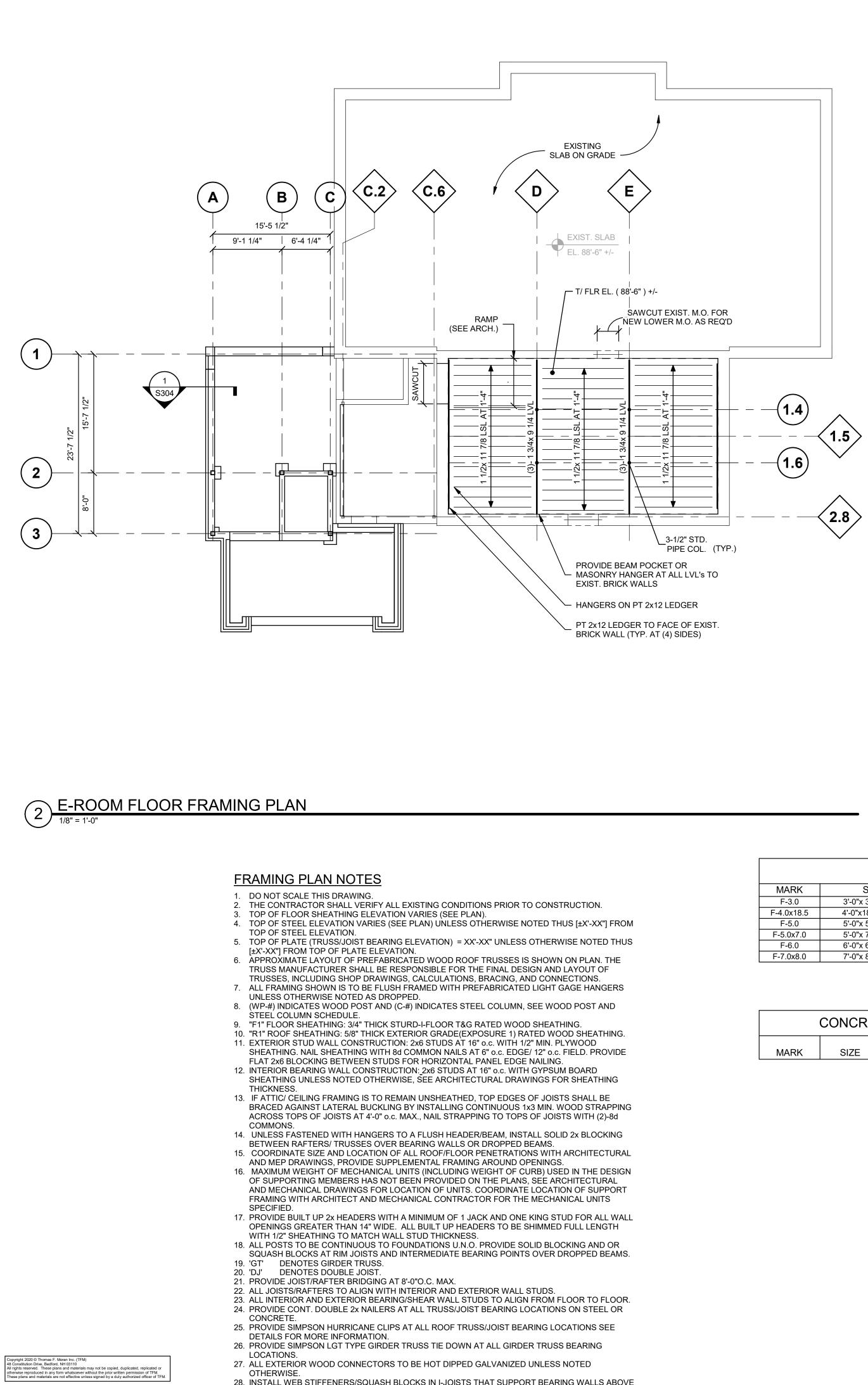
FOR BUDGET ONLY

**S003** 

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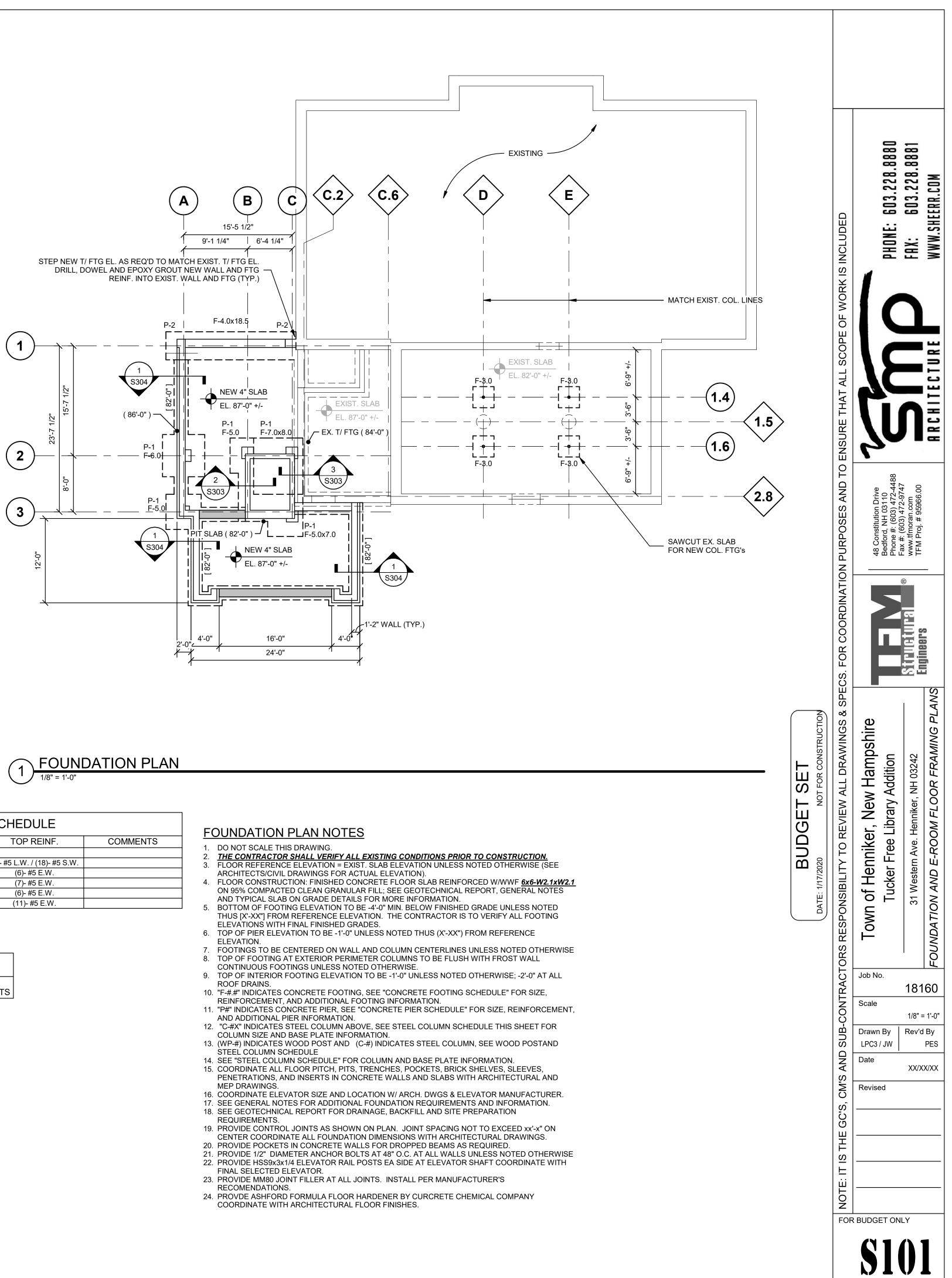
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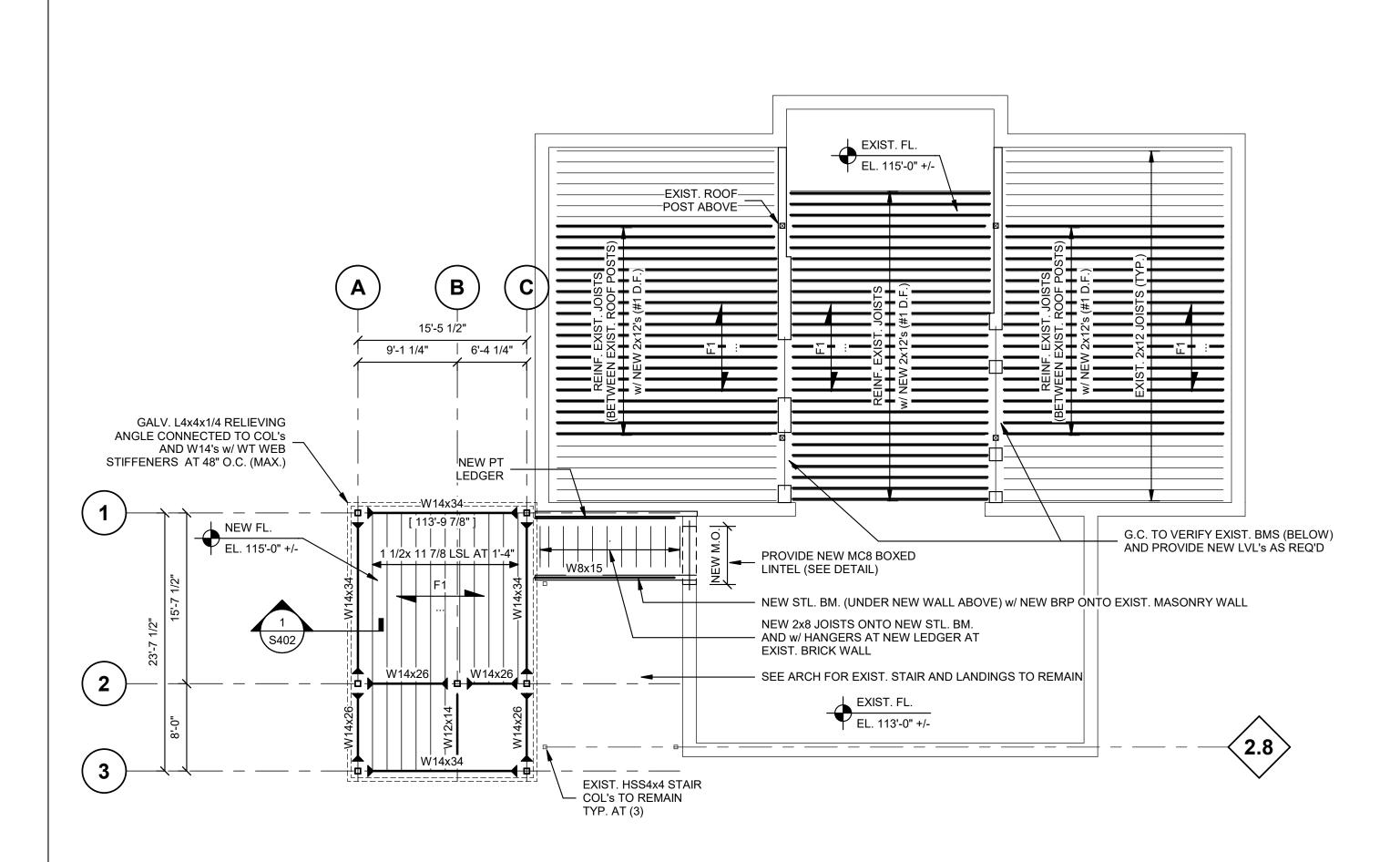
IN ACCORDANCE WITH I-JOIST MANUFACTURER'S SPECIFICATIONS

29. PROVIDE MINIMUM OF (4) 2x POSTS IN WALLS AT BEAM/GIRDER TRUSS BEARING LOCATIONS.

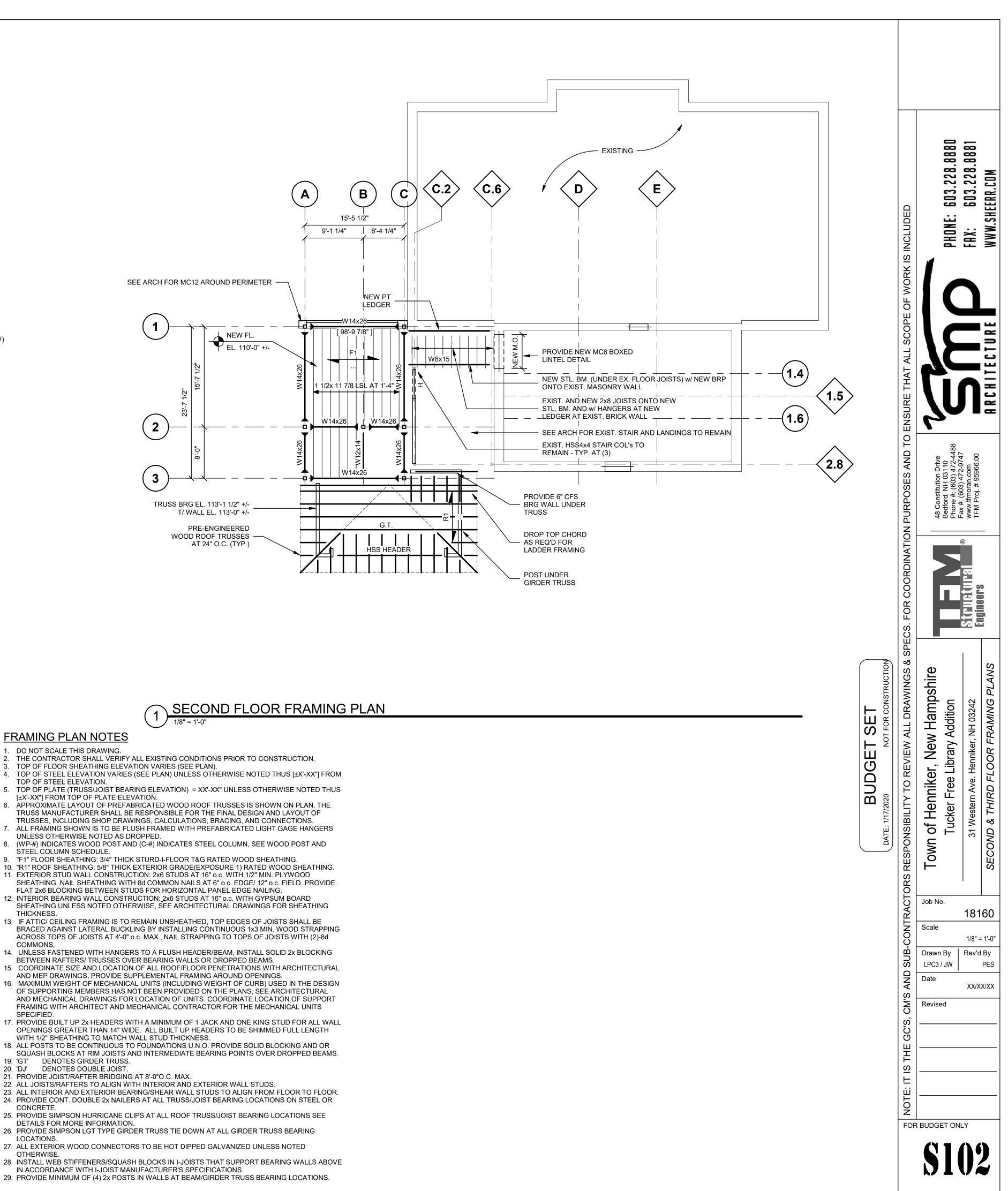


	CONCRETE FOOTING SCHEDULE							
MARK	SIZE	BOT. REINF.	TOP REINF.	COMMENTS				
F-3.0	3'-0"x 3'-0"x 1'-0"	(4)- #5 E.W.						
F-4.0x18.5	4'-0"x18'-6"x 1'-0"	(4)- #5 L.W. / (18)- #5 S.W.	(4)- #5 L.W. / (18)- #5 S.W.					
F-5.0	5'-0"x 5'-0"x 1'-0"	(6)- #5 E.W.	(6)- #5 E.W.					
F-5.0x7.0	5'-0"x 7'-0"x 1'-0"	(7)- #5 E.W.	(7)- #5 E.W.					
F-6.0	6'-0"x 6'-0"x 1'-0"	(6)- #5 E.W.	(6)- #5 E.W.					
F-7.0x8.0	7'-0"x 8'-0"x 1'-6"	(11)- #5 E.W.	(11)- #5 E.W.					

	CONCRET	TE PIER S	CHEDULE	
MARK	SIZE	VERT. REINF.	TIE REINF.	COMMENTS

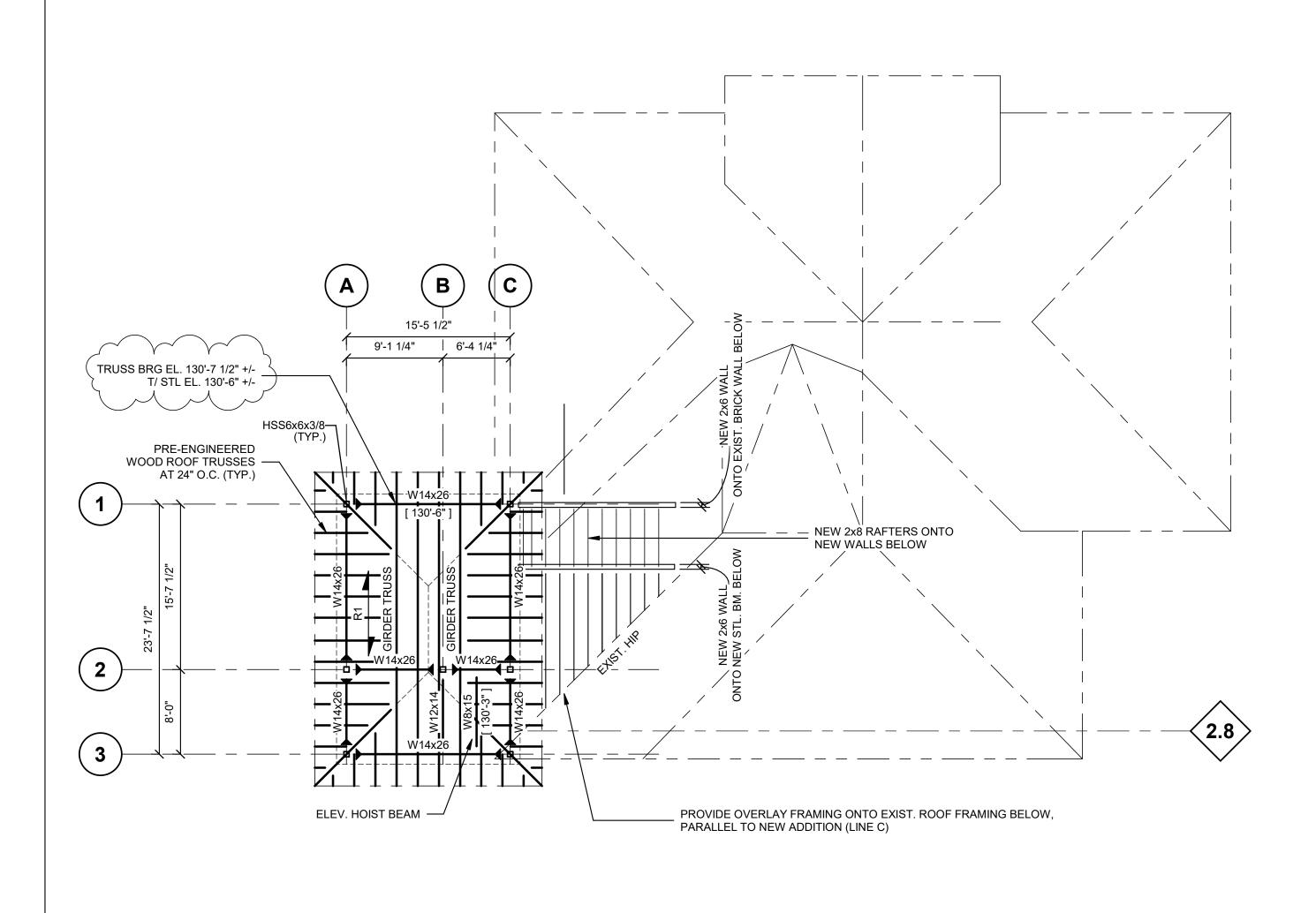


# 2 THIRD FLOOR FRAMING PLAN

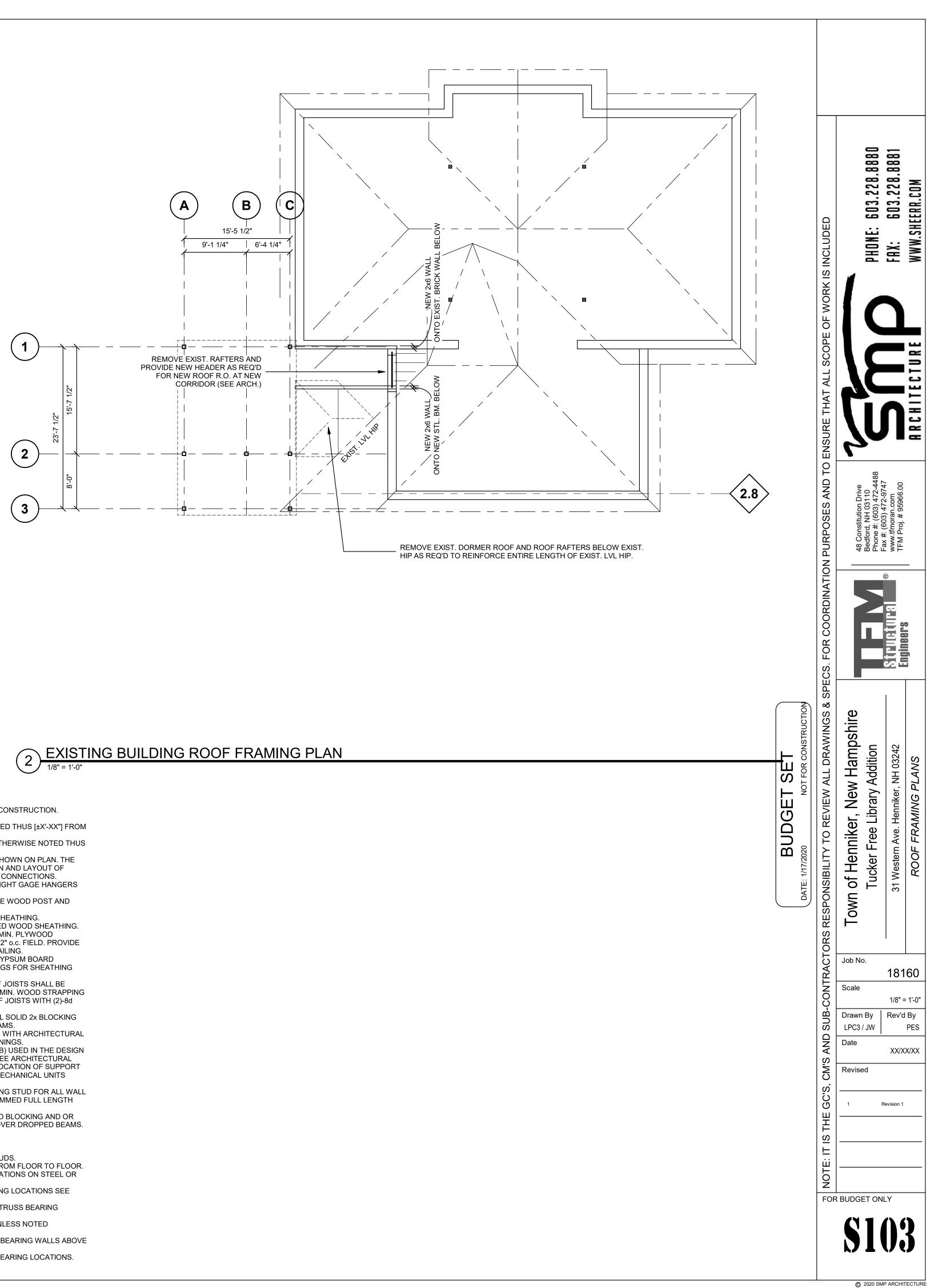


FRAMING PLAN NOTES

- 1. DO NOT SCALE THIS DRAWING. 2. THE CONTRACTOR SHALL VERIFY ALL EXISTING CONDITIONS PRIOR TO CONSTRUCTION.
- 3. TOP OF FLOOR SHEATHING ELEVATION VARIES (SEE PLAN).
- TOP OF STEEL ELEVATION. 5. TOP OF PLATE (TRUSS/JOIST BEARING ELEVATION) = XX'-XX" UNLESS OTHERWISE NOTED THUS [±X'-XX"] FROM TOP OF PLATE ELEVATION.
- 6. APPROXIMATE LAYOUT OF PREFABRICATED WOOD ROOF TRUSSES IS SHOWN ON PLAN. THE
- TRUSSES, INCLUDING SHOP DRAWINGS, CALCULATIONS, BRACING, AND CONNECTIONS.
- 7. ALL FRAMING SHOWN IS TO BE FLUSH FRAMED WITH PREFABRICATED LIGHT GAGE HANGERS UNLESS OTHERWISE NOTED AS DROPPED. 8. (WP-#) INDICATES WOOD POST AND (C-#) INDICATES STEEL COLUMN, SEE WOOD POST AND
- STEEL COLUMN SCHEDULE.
- 10. "R1" ROOF SHEATHING: 5/8" THICK EXTERIOR GRADE(EXPOSURE 1) RATED WOOD SHEATHING. 11. EXTERIOR STUD WALL CONSTRUCTION: 2x6 STUDS AT 16" o.c. WITH 1/2" MIN. PLYWOOD SHEATHING. NAIL SHEATHING WITH 8d COMMON NAILS AT 6" o.c. EDGE/ 12" o.c. FIELD. PROVIDE
- FLAT 2x6 BLOCKING BETWEEN STUDS FOR HORIZONTAL PANEL EDGE NAILING. 12. INTERIOR BEARING WALL CONSTRUCTION: 2x6 STUDS AT 16" o.c. WITH GYPSUM BOARD SHEATHING UNLESS NOTED OTHERWISE, SEE ARCHITECTURAL DRAWINGS FOR SHEATHING
- THICKNESS. 13. IF ATTIC/ CEILING FRAMING IS TO REMAIN UNSHEATHED, TOP EDGES OF JOISTS SHALL BE BRACED AGAINST LATERAL BUCKLING BY INSTALLING CONTINUOUS 1x3 MIN. WOOD STRAPPING ACROSS TOPS OF JOISTS AT 4'-0" o.c. MAX., NAIL STRAPPING TO TOPS OF JOISTS WITH (2)-8d COMMONS.
- 14. UNLESS FASTENED WITH HANGERS TO A FLUSH HEADER/BEAM, INSTALL SOLID 2x BLOCKING BETWEEN RAFTERS/ TRUSSES OVER BEARING WALLS OR DROPPED BEAMS.
- AND MEP DRAWINGS, PROVIDE SUPPLEMENTAL FRAMING AROUND OPENINGS. 16. MAXIMUM WEIGHT OF MECHANICAL UNITS (INCLUDING WEIGHT OF CURB) USED IN THE DESIGN
- AND MECHANICAL DRAWINGS FOR LOCATION OF UNITS. COORDINATE LOCATION OF SUPPORT FRAMING WITH ARCHITECT AND MECHANICAL CONTRACTOR FOR THE MECHANICAL UNITS SPECIFIED.
- OPENINGS GREATER THAN 14" WIDE. ALL BUILT UP HEADERS TO BE SHIMMED FULL LENGTH WITH 1/2" SHEATHING TO MATCH WALL STUD THICKNESS.
- SQUASH BLOCKS AT RIM JOISTS AND INTERMEDIATE BEARING POINTS OVER DROPPED BEAMS. 19. 'GT' DENOTES GIRDER TRUSS.
- 20. 'DJ' DENOTES DOUBLE JOIST. 21. PROVIDE JOIST/RAFTER BRIDGING AT 8'-0"O.C. MAX.
- 22. ALL JOISTS/RAFTERS TO ALIGN WITH INTERIOR AND EXTERIOR WALL STUDS. 23. ALL INTERIOR AND EXTERIOR BEARING/SHEAR WALL STUDS TO ALIGN FROM FLOOR TO FLOOR. 24. PROVIDE CONT. DOUBLE 2x NAILERS AT ALL TRUSS/JOIST BEARING LOCATIONS ON STEEL OR
- CONCRETE. 25. PROVIDE SIMPSON HURRICANE CLIPS AT ALL ROOF TRUSS/JOIST BEARING LOCATIONS SEE
- DETAILS FOR MORE INFORMATION. 26. PROVIDE SIMPSON LGT TYPE GIRDER TRUSS TIE DOWN AT ALL GIRDER TRUSS BEARING
- LOCATIONS. 27. ALL EXTERIOR WOOD CONNECTORS TO BE HOT DIPPED GALVANIZED UNLESS NOTED
- OTHERWISE.
- 28. INSTALL WEB STIFFENERS/SQUASH BLOCKS IN I-JOISTS THAT SUPPORT BEARING WALLS ABOVE IN ACCORDANCE WITH I-JOIST MANUFACTURER'S SPECIFICATIONS
- 29. PROVIDE MINIMUM OF (4) 2x POSTS IN WALLS AT BEAM/GIRDER TRUSS BEARING LOCATIONS.

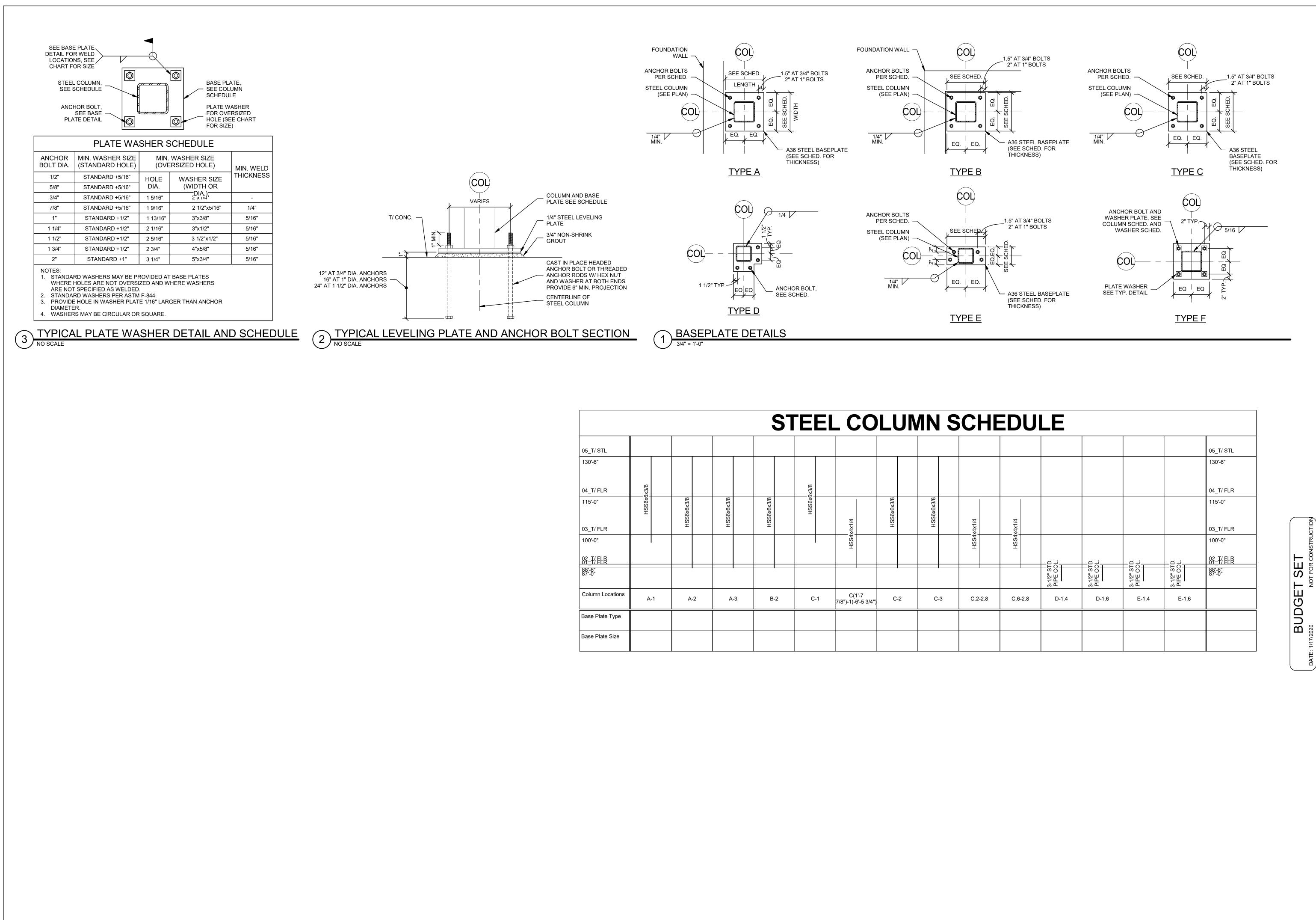


1 ROOF FRAMING PLAN



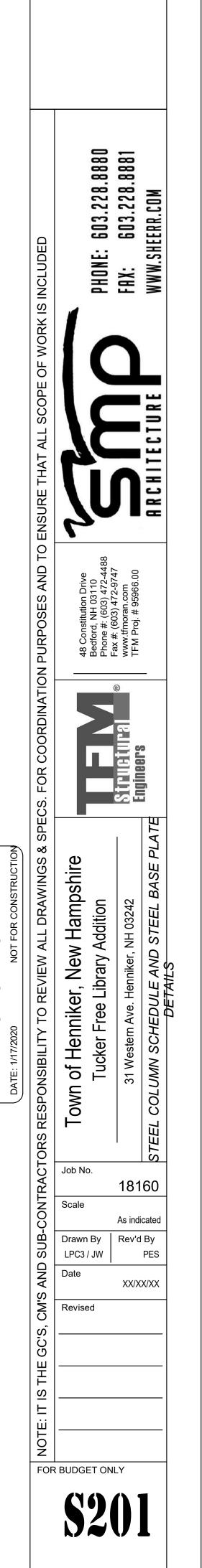
### FRAMING PLAN NOTES

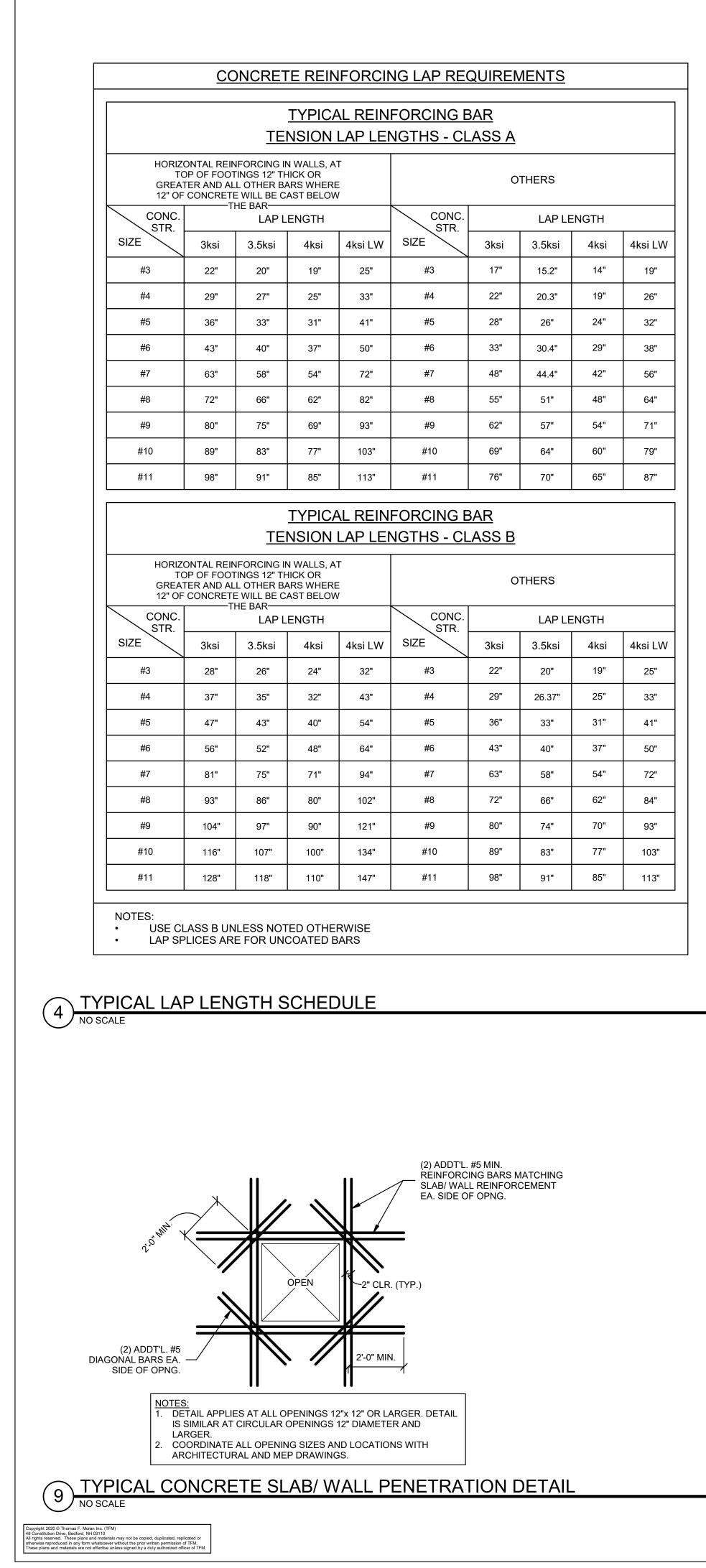
- 1. DO NOT SCALE THIS DRAWING. THE CONTRACTOR SHALL VERIFY ALL EXISTING CONDITIONS PRIOR TO CONSTRUCTION.
- TOP OF FLOOR SHEATHING ELEVATION VARIES (SEE PLAN).
- 4. TOP OF STEEL ELEVATION VARIES (SEE PLAN) UNLESS OTHERWISE NOTED THUS [±X'-XX"] FROM TOP OF STEEL ELEVATION.
- 5. TOP OF PLATE (TRUSS/JOIST BEARING ELEVATION) = XX'-XX" UNLESS OTHERWISE NOTED THUS
- [±X'-XX"] FROM TOP OF PLATE ELEVATION. 6. APPROXIMATE LAYOUT OF PREFABRICATED WOOD ROOF TRUSSES IS SHOWN ON PLAN. THE
- TRUSS MANUFACTURER SHALL BE RESPONSIBLE FOR THE FINAL DESIGN AND LAYOUT OF
- TRUSSES, INCLUDING SHOP DRAWINGS, CALCULATIONS, BRACING, AND CONNECTIONS. 7. ALL FRAMING SHOWN IS TO BE FLUSH FRAMED WITH PREFABRICATED LIGHT GAGE HANGERS
- UNLESS OTHERWISE NOTED AS DROPPED. 8. (WP-#) INDICATES WOOD POST AND (C-#) INDICATES STEEL COLUMN, SEE WOOD POST AND
- STEEL COLUMN SCHEDULE.
- 9. "F1" FLOOR SHEATHING: 3/4" THICK STURD-I-FLOOR T&G RATED WOOD SHEATHING. 10. "R1" ROOF SHEATHING: 5/8" THICK EXTERIOR GRADE(EXPOSURE 1) RATED WOOD SHEATHING. 11. EXTERIOR STUD WALL CONSTRUCTION: 2x6 STUDS AT 16" o.c. WITH 1/2" MIN. PLYWOOD SHEATHING. NAIL SHEATHING WITH 8d COMMON NAILS AT 6" o.c. EDGE/ 12" o.c. FIELD. PROVIDE
- FLAT 2x6 BLOCKING BETWEEN STUDS FOR HORIZONTAL PANEL EDGE NAILING. 12. INTERIOR BEARING WALL CONSTRUCTION: 2x6 STUDS AT 16" o.c. WITH GYPSUM BOARD SHEATHING UNLESS NOTED OTHERWISE, SEE ARCHITECTURAL DRAWINGS FOR SHEATHING
- THICKNESS. 13. IF ATTIC/ CEILING FRAMING IS TO REMAIN UNSHEATHED, TOP EDGES OF JOISTS SHALL BE
- BRACED AGAINST LATERAL BUCKLING BY INSTALLING CONTINUOUS 1x3 MIN. WOOD STRAPPING ACROSS TOPS OF JOISTS AT 4'-0" o.c. MAX., NAIL STRAPPING TO TOPS OF JOISTS WITH (2)-8d COMMONS.
- 14. UNLESS FASTENED WITH HANGERS TO A FLUSH HEADER/BEAM, INSTALL SOLID 2x BLOCKING BETWEEN RAFTERS/ TRUSSES OVER BEARING WALLS OR DROPPED BEAMS.
- 15. COORDINATE SIZE AND LOCATION OF ALL ROOF/FLOOR PENETRATIONS WITH ARCHITECTURAL
- AND MEP DRAWINGS, PROVIDE SUPPLEMENTAL FRAMING AROUND OPENINGS. 16. MAXIMUM WEIGHT OF MECHANICAL UNITS (INCLUDING WEIGHT OF CURB) USED IN THE DESIGN OF SUPPORTING MEMBERS HAS NOT BEEN PROVIDED ON THE PLANS, SEE ARCHITECTURAL AND MECHANICAL DRAWINGS FOR LOCATION OF UNITS. COORDINATE LOCATION OF SUPPORT FRAMING WITH ARCHITECT AND MECHANICAL CONTRACTOR FOR THE MECHANICAL UNITS SPECIFIED.
- 17. PROVIDE BUILT UP 2x HEADERS WITH A MINIMUM OF 1 JACK AND ONE KING STUD FOR ALL WALL OPENINGS GREATER THAN 14" WIDE. ALL BUILT UP HEADERS TO BE SHIMMED FULL LENGTH WITH 1/2" SHEATHING TO MATCH WALL STUD THICKNESS.
- 18. ALL POSTS TO BE CONTINUOUS TO FOUNDATIONS U.N.O. PROVIDE SOLID BLOCKING AND OR SQUASH BLOCKS AT RIM JOISTS AND INTERMEDIATE BEARING POINTS OVER DROPPED BEAMS. 19. 'GT' DENOTES GIRDER TRUSS.
- 20. 'DJ' DENOTES DOUBLE JOIST. 21. PROVIDE JOIST/RAFTER BRIDGING AT 8'-0"O.C. MAX.
- 22. ALL JOISTS/RAFTERS TO ALIGN WITH INTERIOR AND EXTERIOR WALL STUDS.
- 23. ALL INTERIOR AND EXTERIOR BEARING/SHEAR WALL STUDS TO ALIGN FROM FLOOR TO FLOOR. 24. PROVIDE CONT. DOUBLE 2x NAILERS AT ALL TRUSS/JOIST BEARING LOCATIONS ON STEEL OR
- CONCRETE. 25. PROVIDE SIMPSON HURRICANE CLIPS AT ALL ROOF TRUSS/JOIST BEARING LOCATIONS SEE
- DETAILS FOR MORE INFORMATION.
- 26. PROVIDE SIMPSON LGT TYPE GIRDER TRUSS TIE DOWN AT ALL GIRDER TRUSS BEARING LOCATIONS.
- 27. ALL EXTERIOR WOOD CONNECTORS TO BE HOT DIPPED GALVANIZED UNLESS NOTED
- OTHERWISE. 28. INSTALL WEB STIFFENERS/SQUASH BLOCKS IN I-JOISTS THAT SUPPORT BEARING WALLS ABOVE IN ACCORDANCE WITH I-JOIST MANUFACTURER'S SPECIFICATIONS
- 29. PROVIDE MINIMUM OF (4) 2x POSTS IN WALLS AT BEAM/GIRDER TRUSS BEARING LOCATIONS.

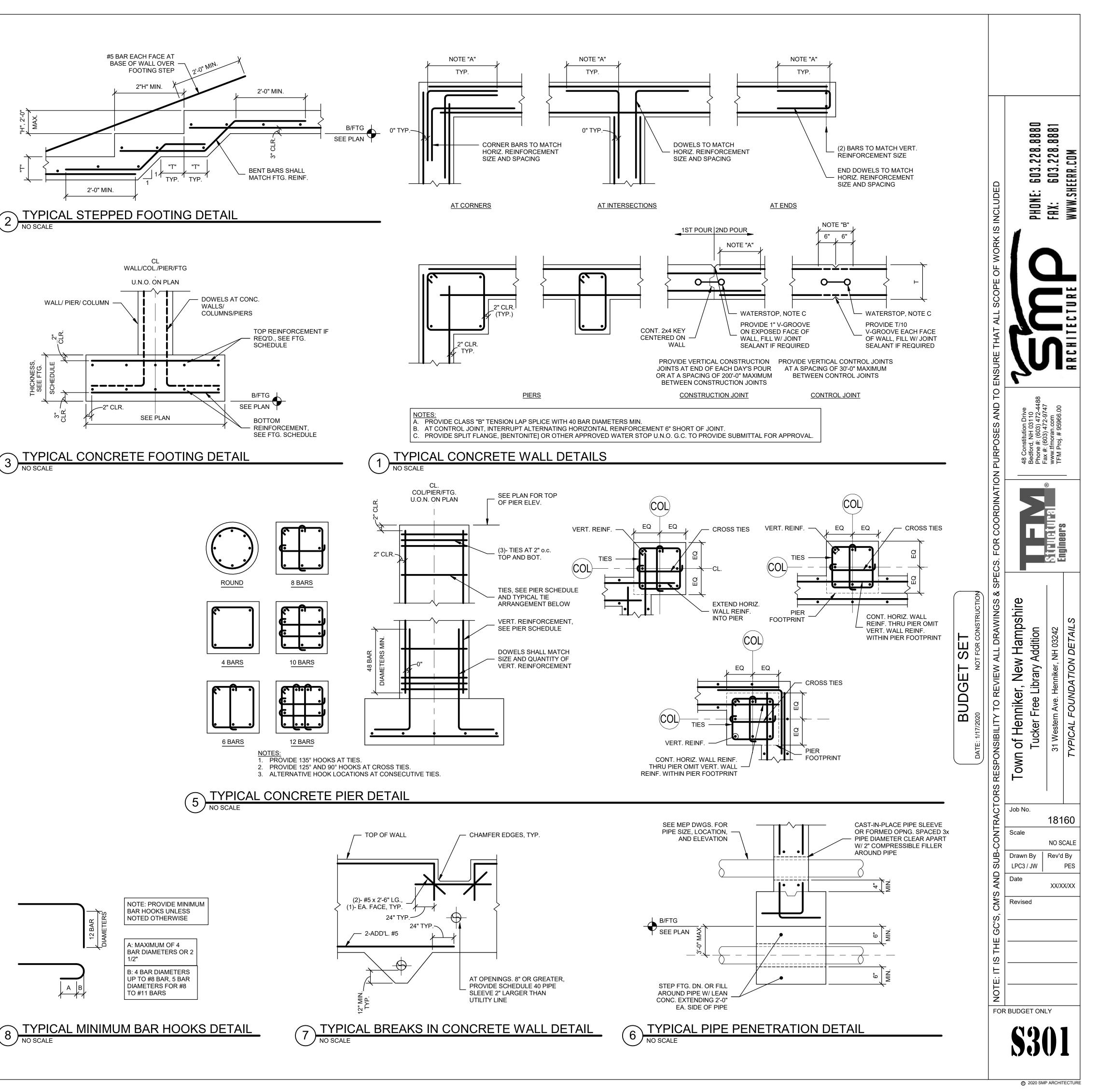


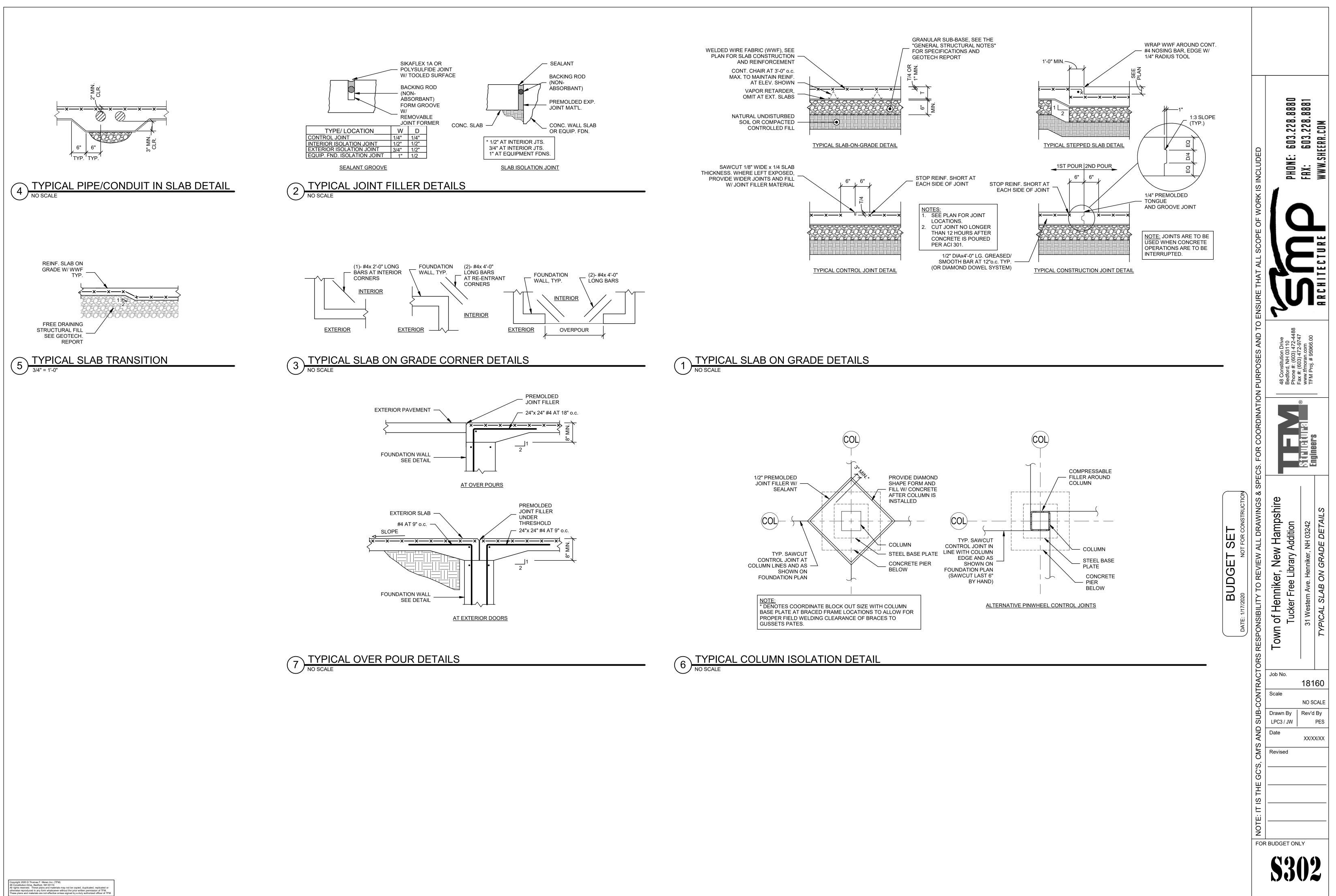
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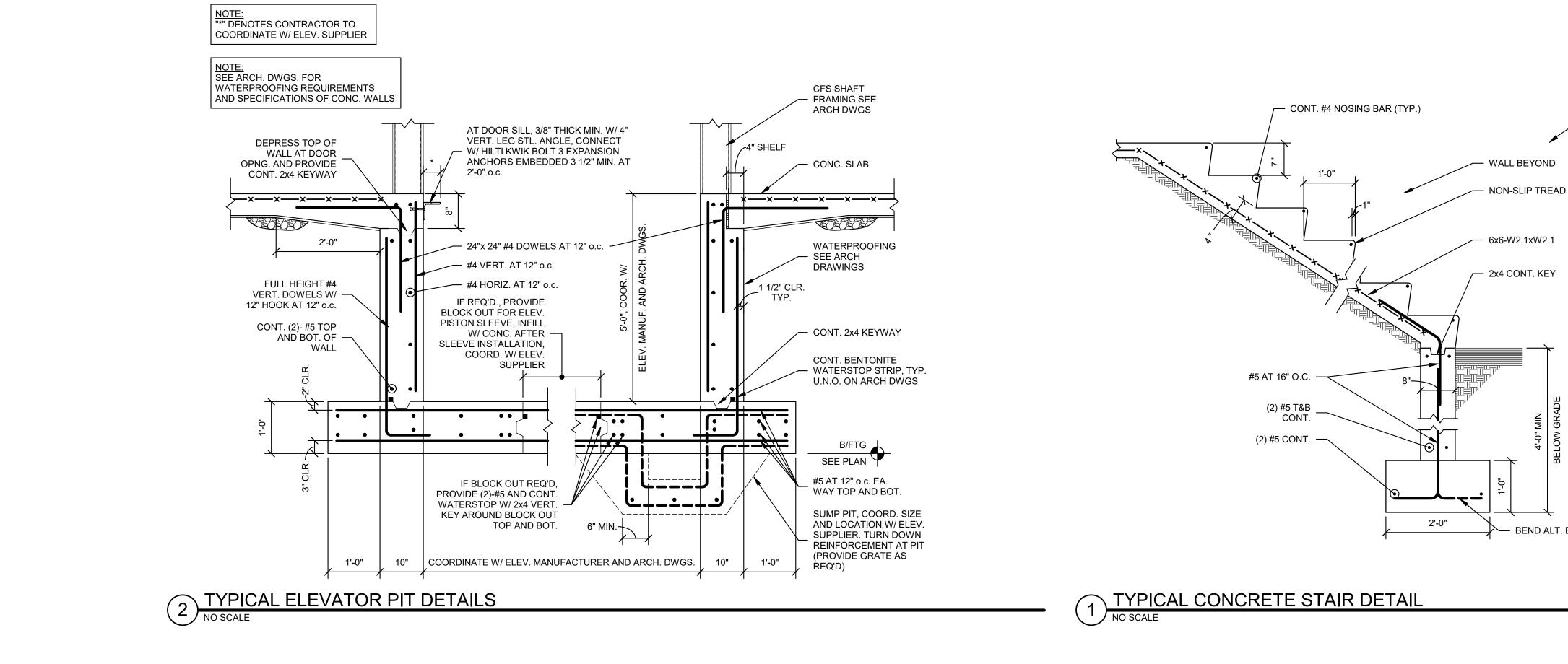
				S	TEE	L CO	LUI	MN S	SCHE	EDU	LE				
05_T/ STL															05_T/ STL
130'-6"															130'-6"
04_T/ FLR	x3/8				ix3/8										04_T/ FLR
115'-0"	HSS6x6x3/8	HSS6x6x3/8	HSS6x6x3/8	6x6x3/8	HSS6x6		HSS6x6x3/8	HSS6x6x3/8							115'-0"
03_T/ FLR		IN SOLUTION OF A CONTRACT OF A	HSS	HSSe		HSS4x4x1/4	SSH	USS H	1/4 1/4	1/4 1/4					03_T/ FLR
100'-0"						HSS4			HSS4	HSS4)					100'-0"
87 <u>-</u> F/FLR															87 <u>-</u> 7/FLR
88'-6" 87'-0"							•	•			3-1/2" S	3-1/2" S	3-1/2" S PIPE C(	3-1/2" S PIPE CC	88'-6" 87'-0"
Column Locations	A-1	A-2	A-3	B-2	C-1	C(1'-7 7/8")-1(-6'-5 3/4")	C-2	C-3	C.2-2.8	C.6-2.8	D-1.4	D-1.6	E-1.4	E-1.6	
Base Plate Type															
Base Plate Size															

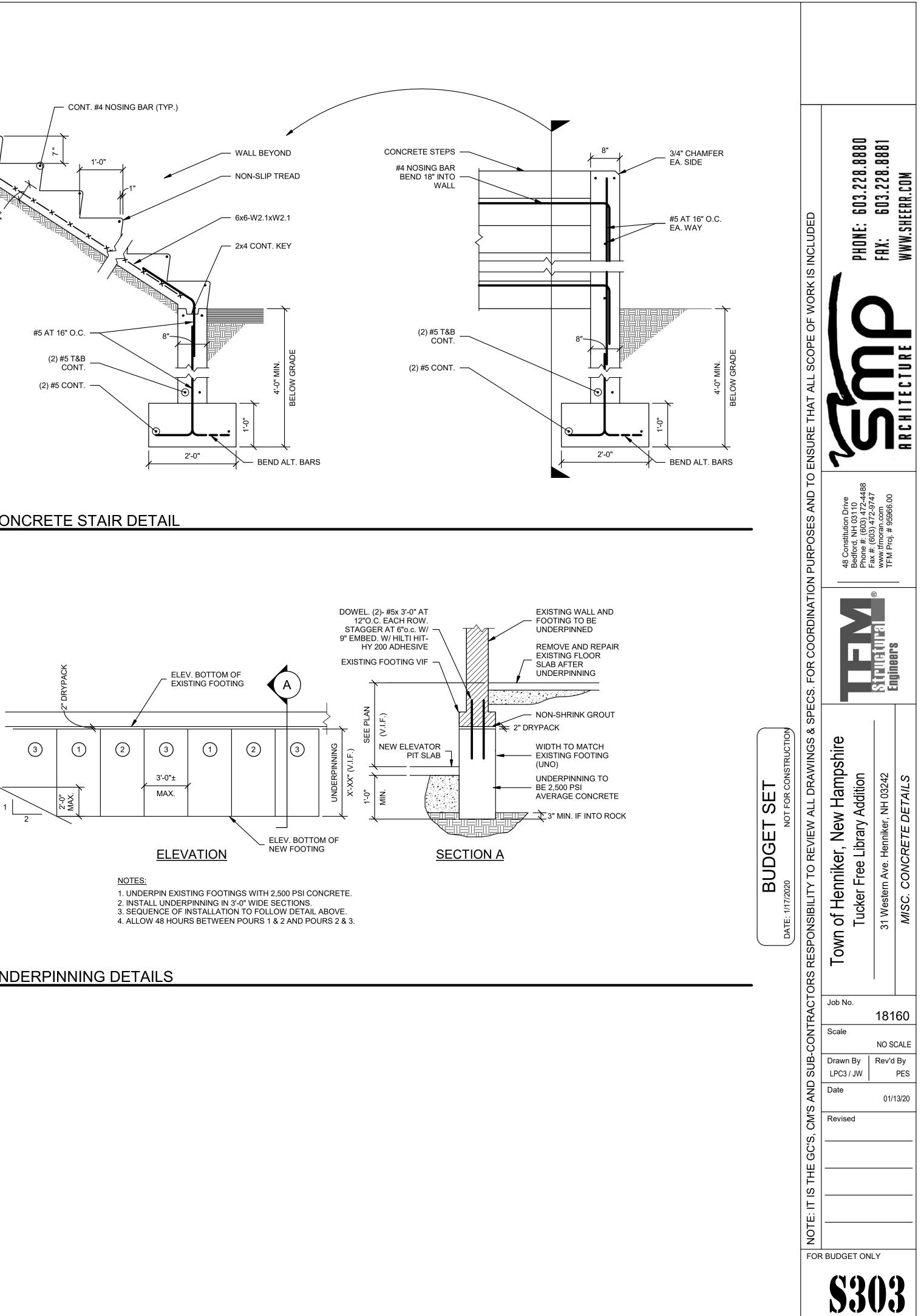








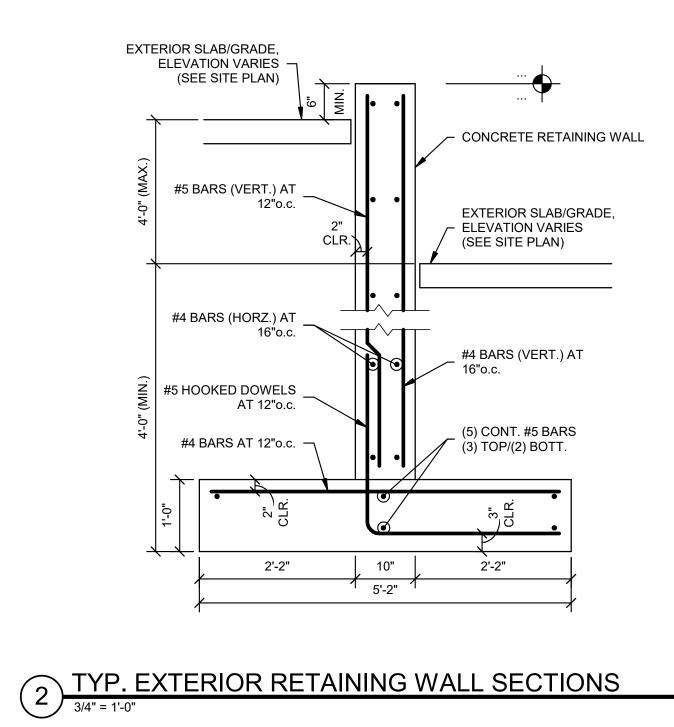


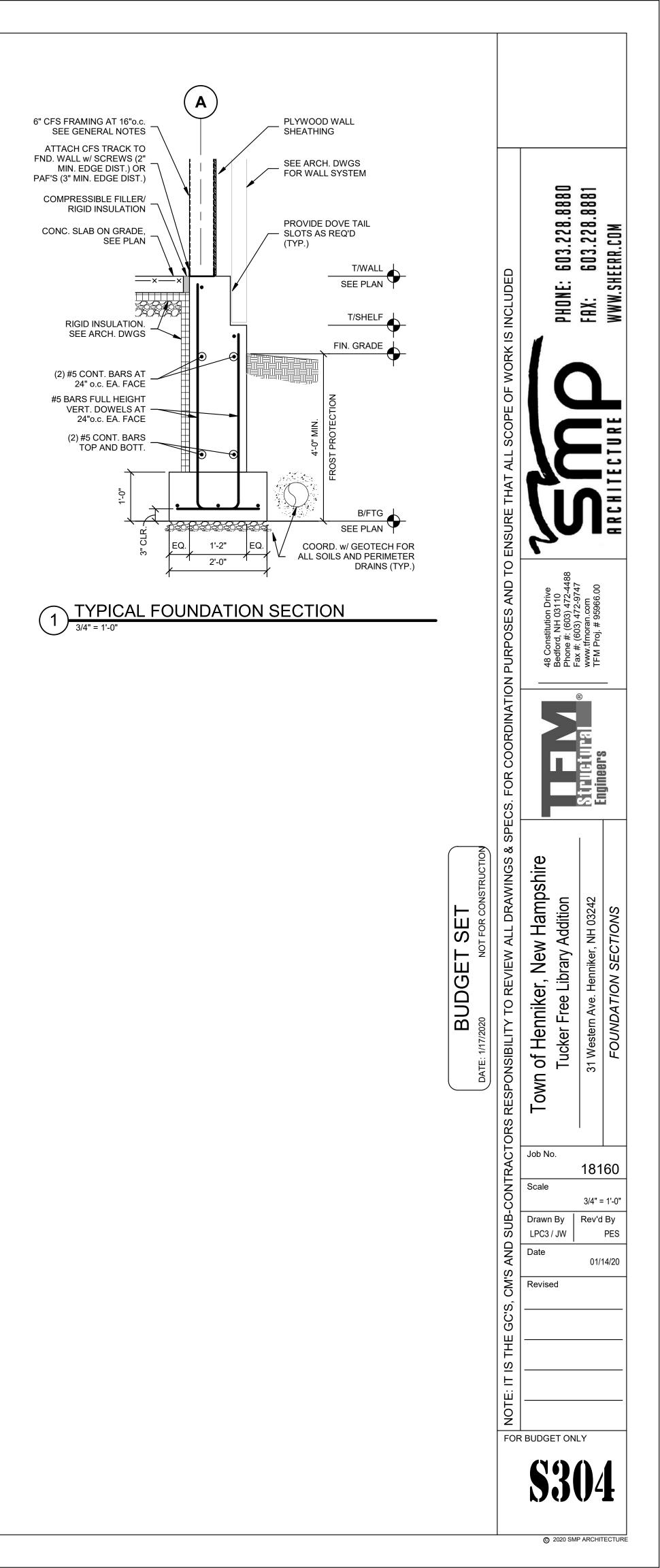


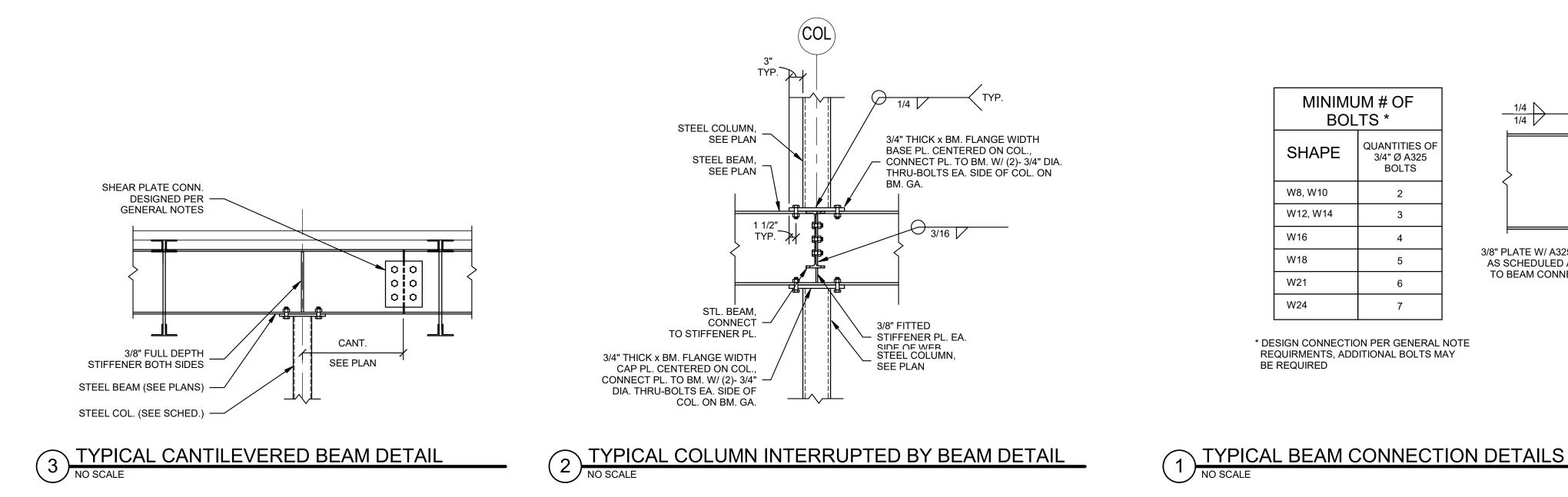


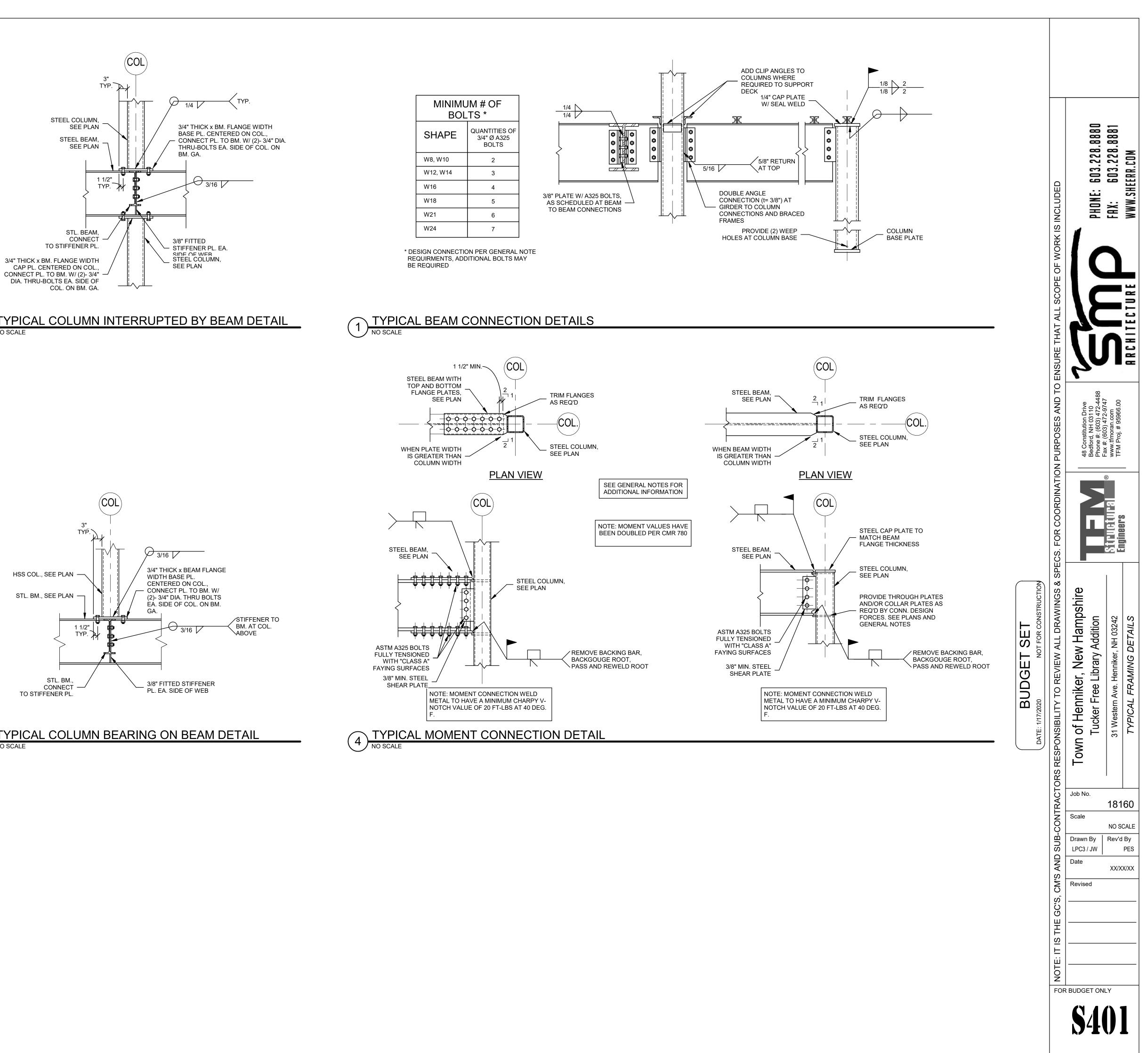
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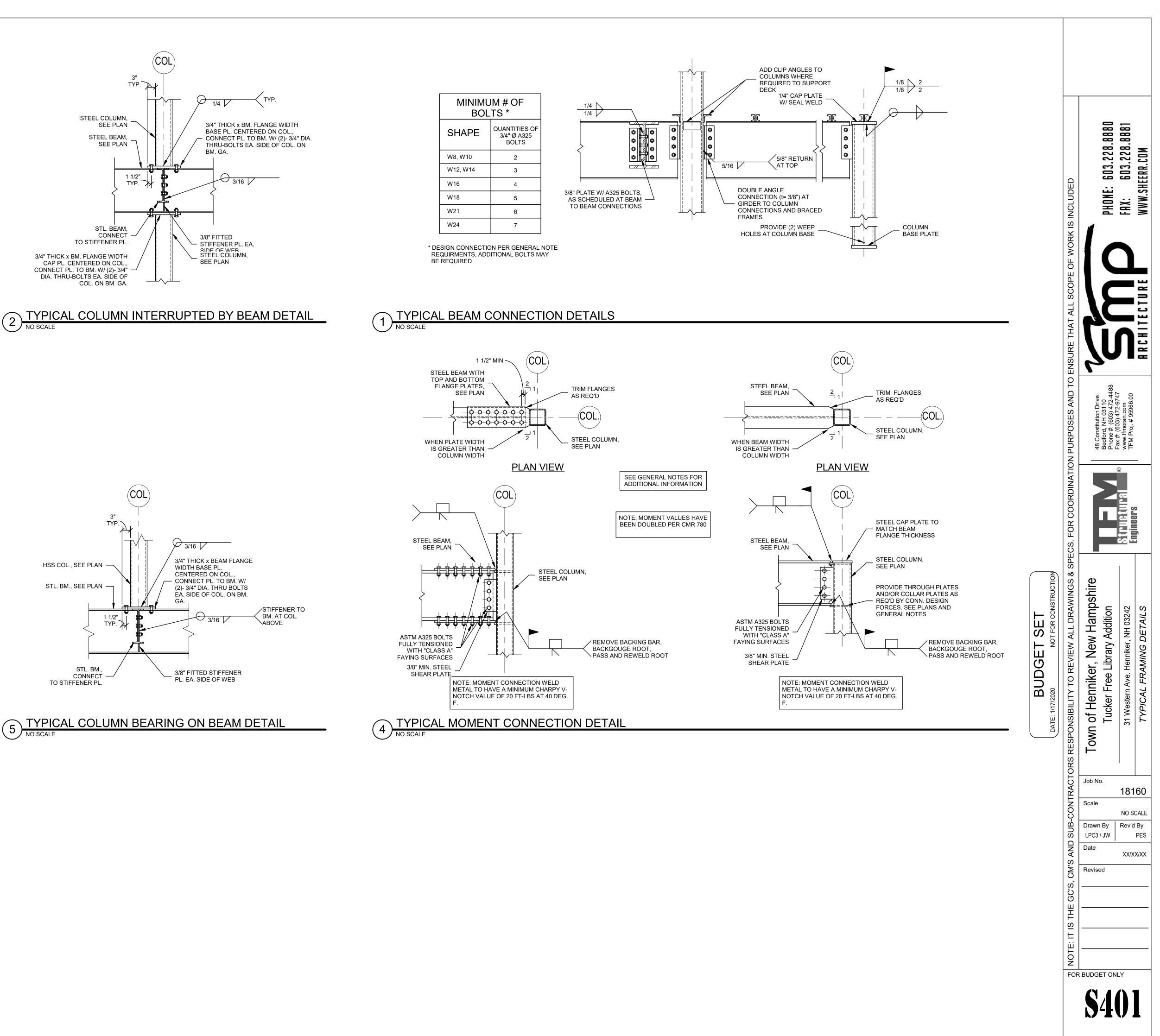
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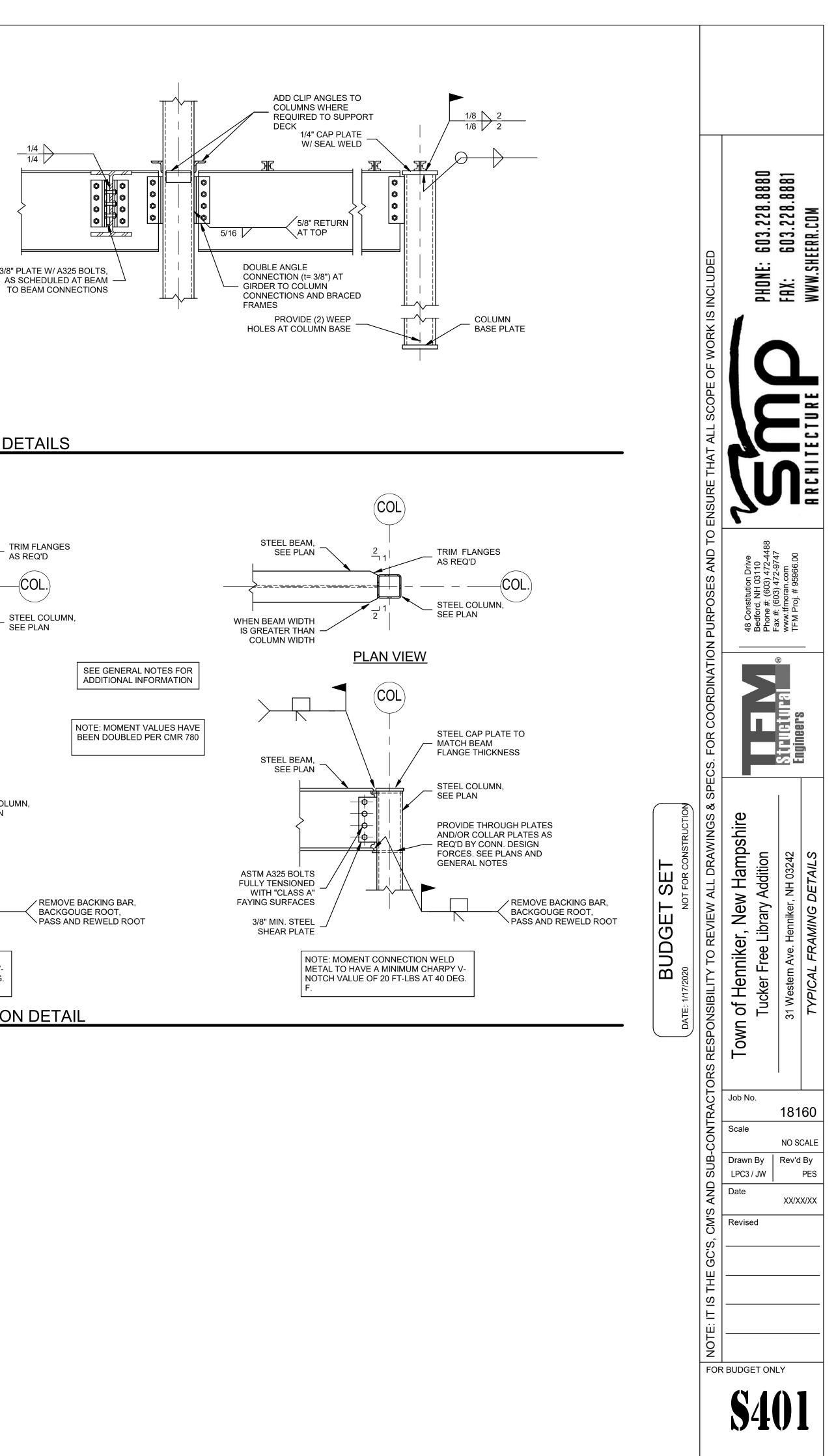




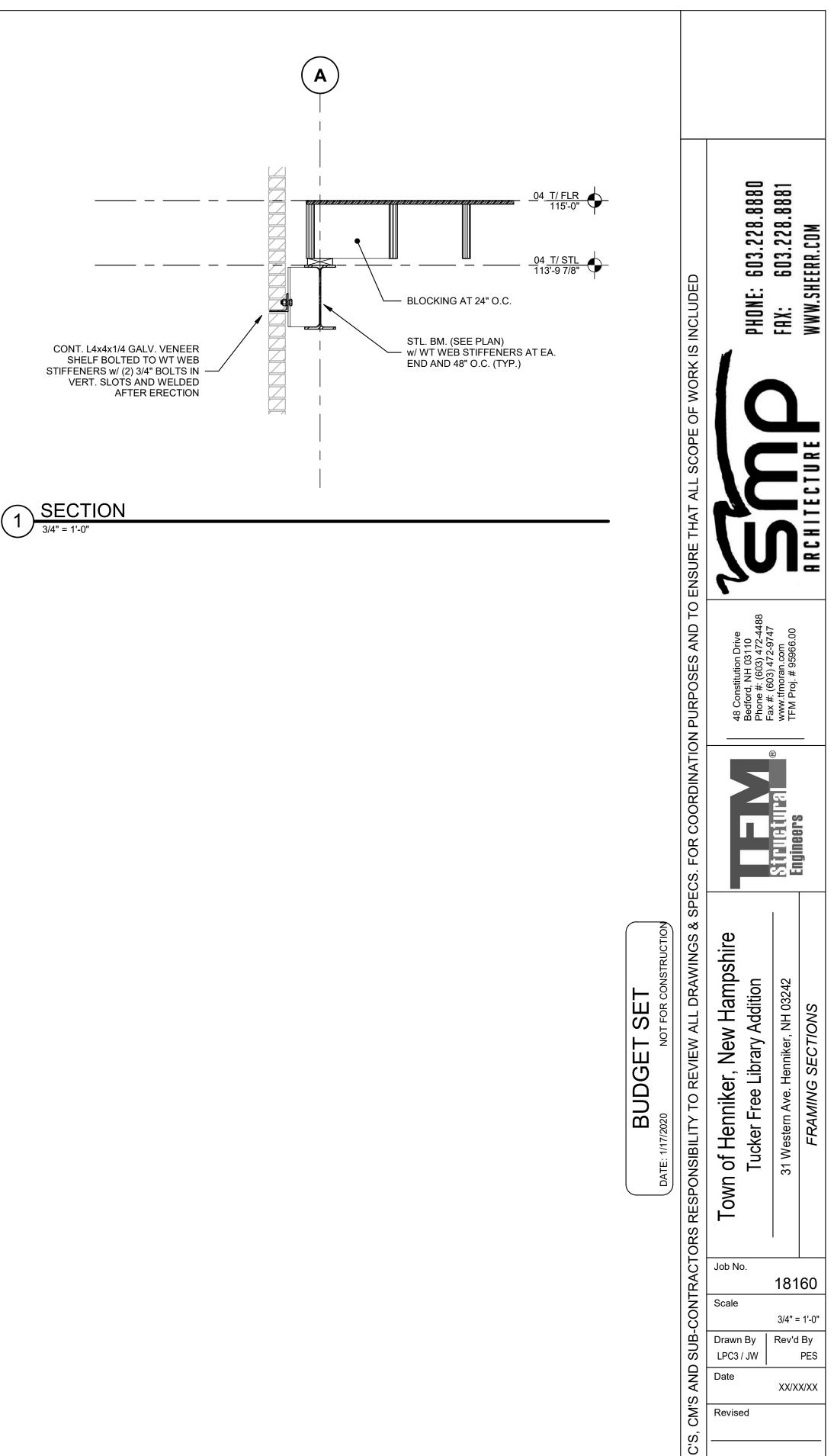


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MINIMUM # OF BOLTS *						
SHAPE	QUANTITIES OF 3/4" Ø A325 BOLTS					
W8, W10	2					
W12, W14	3					
W16	4					
W18	5					
W21	6					
W24	7					



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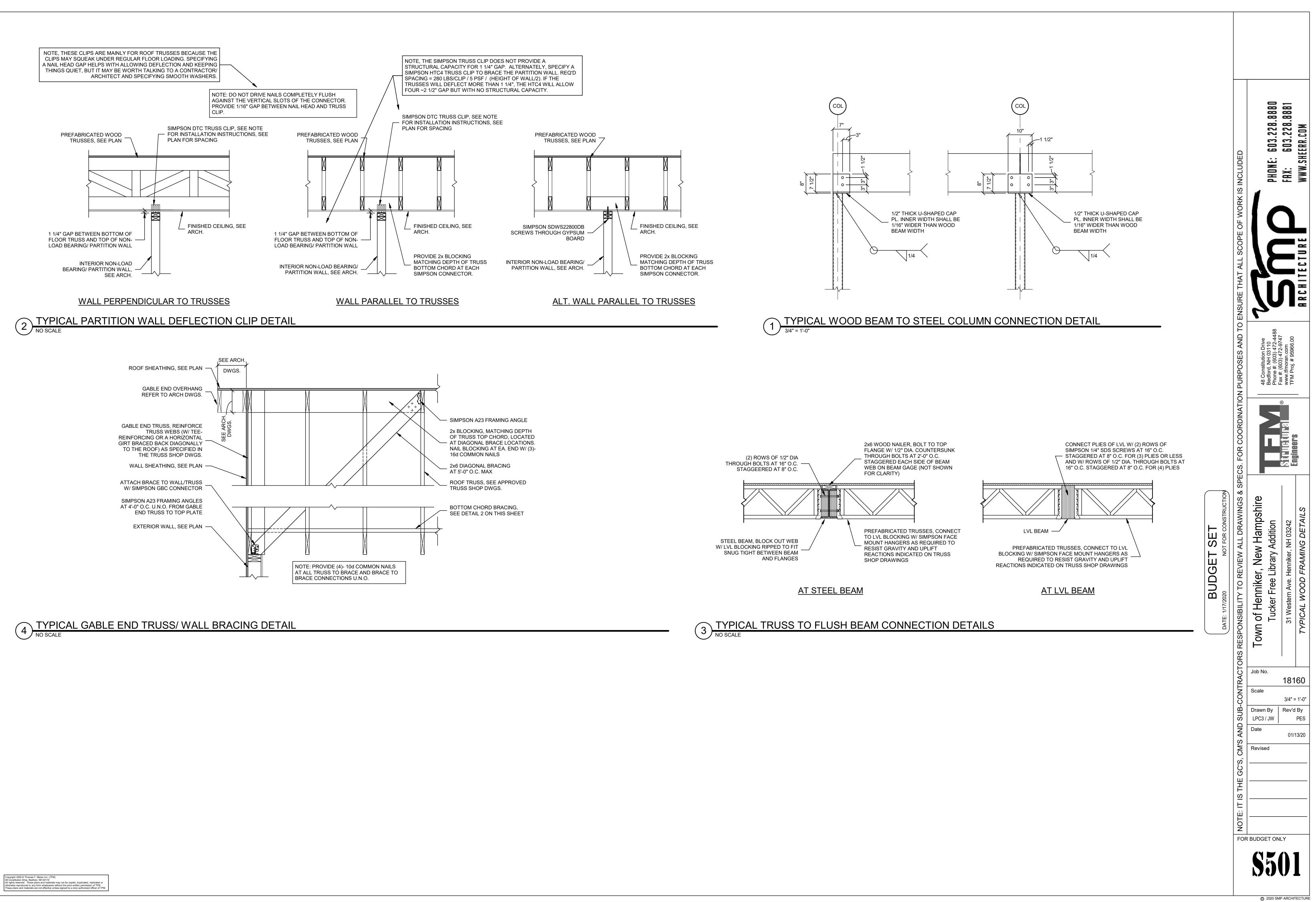
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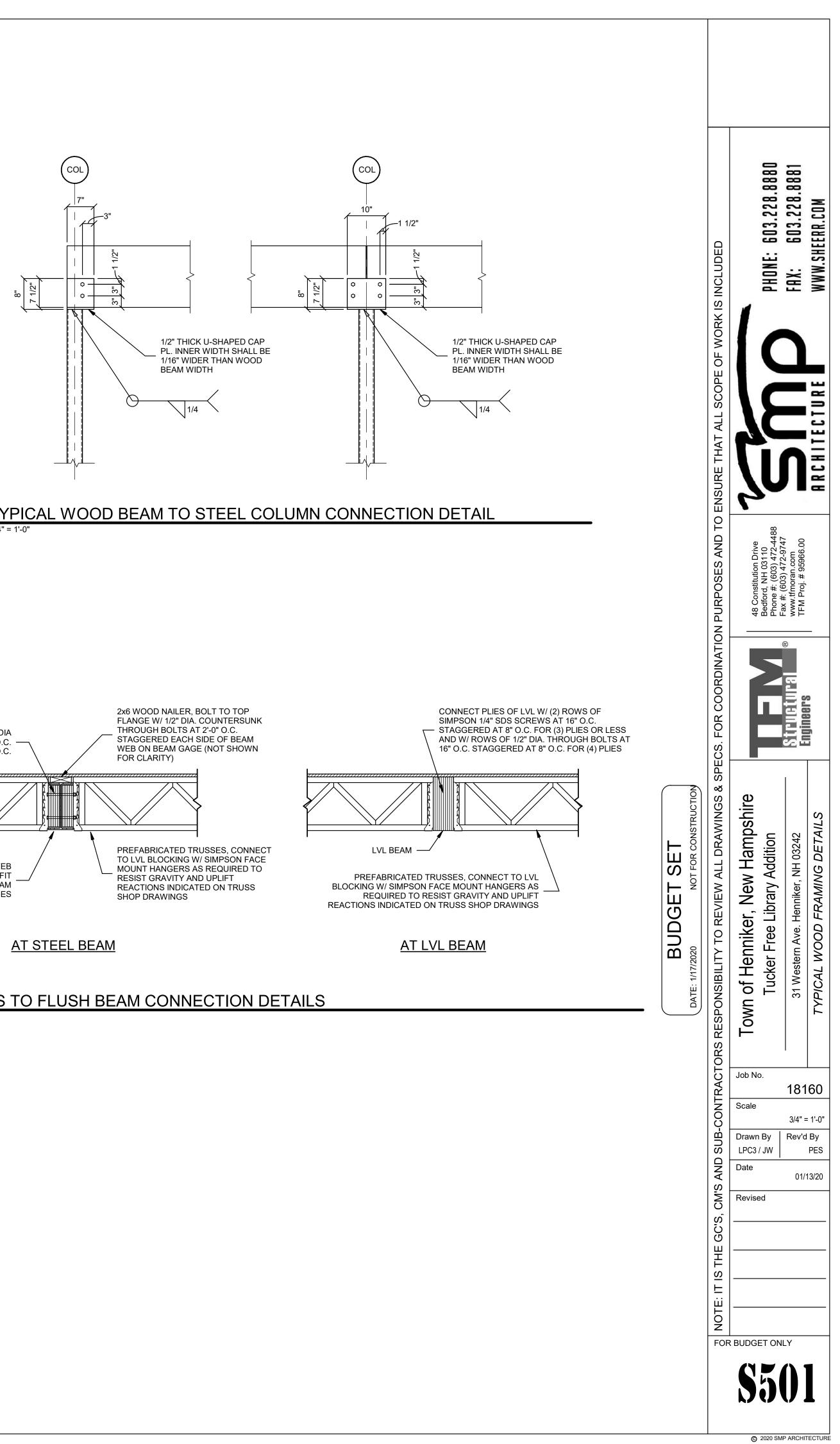
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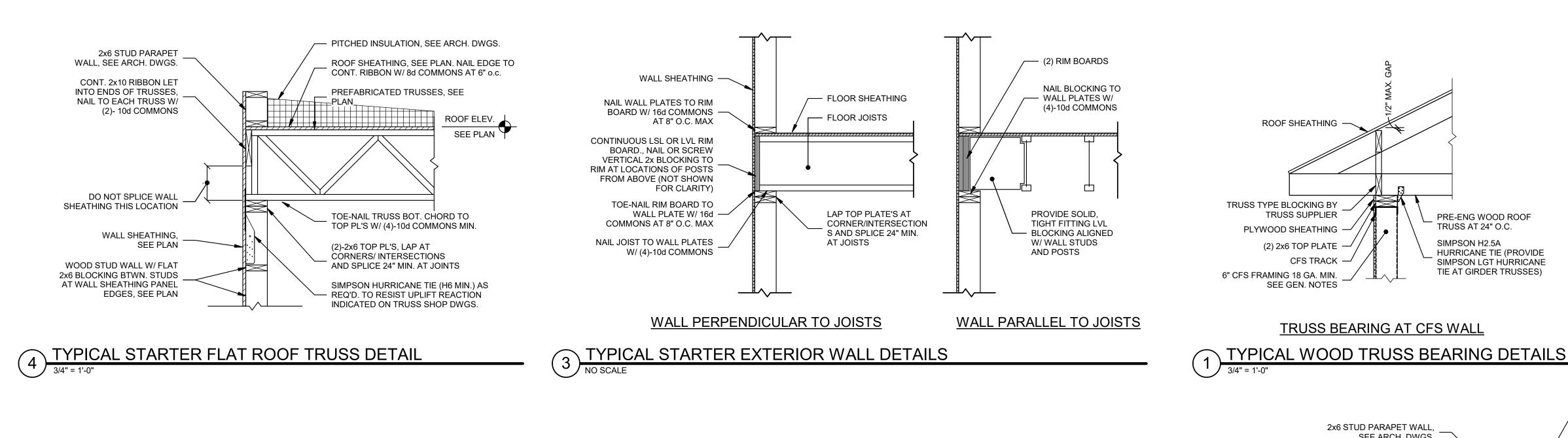
FOR BUDGET ONLY

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**RCHITE** 







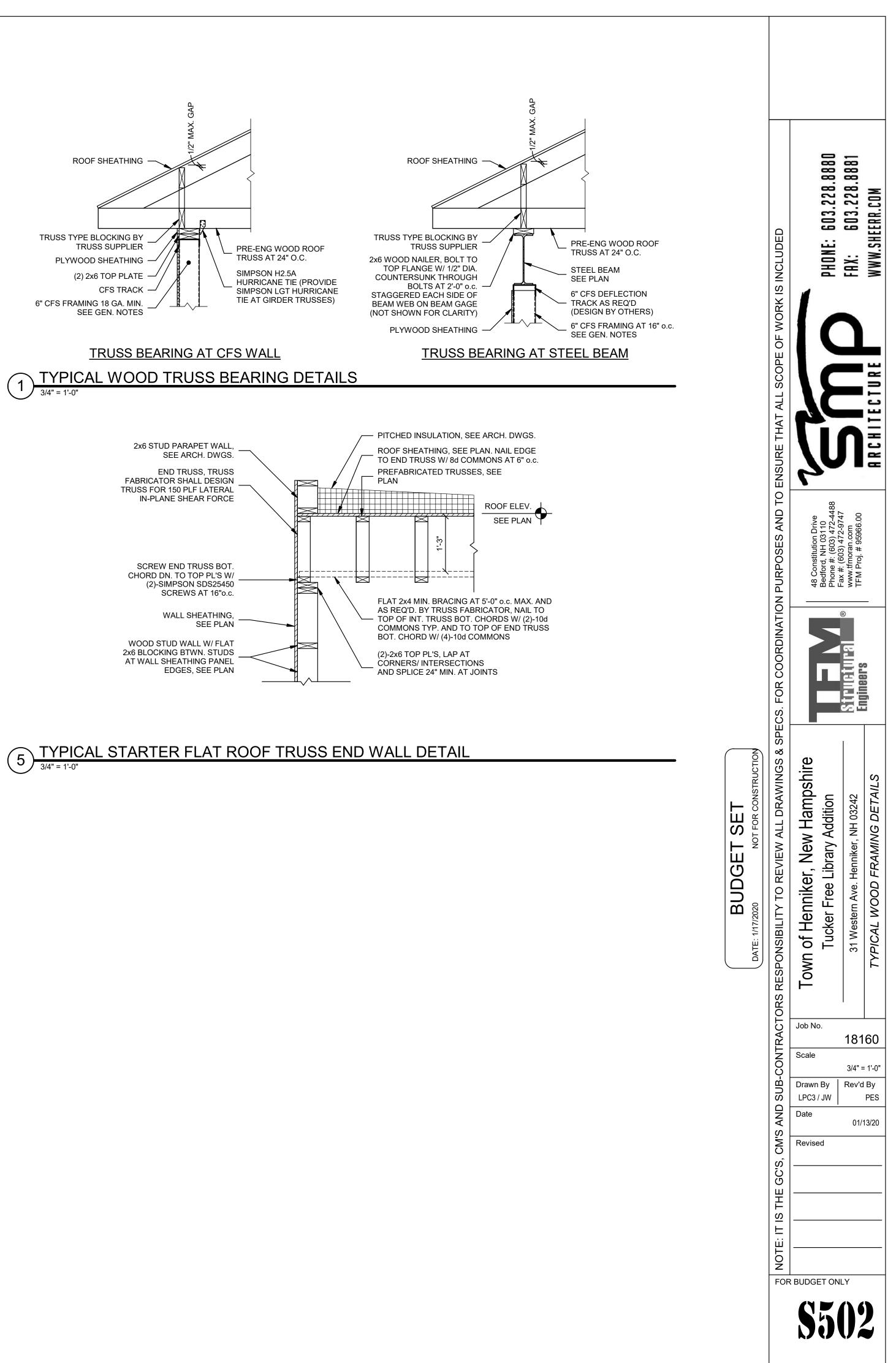
SEE ARCH. DWGS. END TRUSS, TRUSS

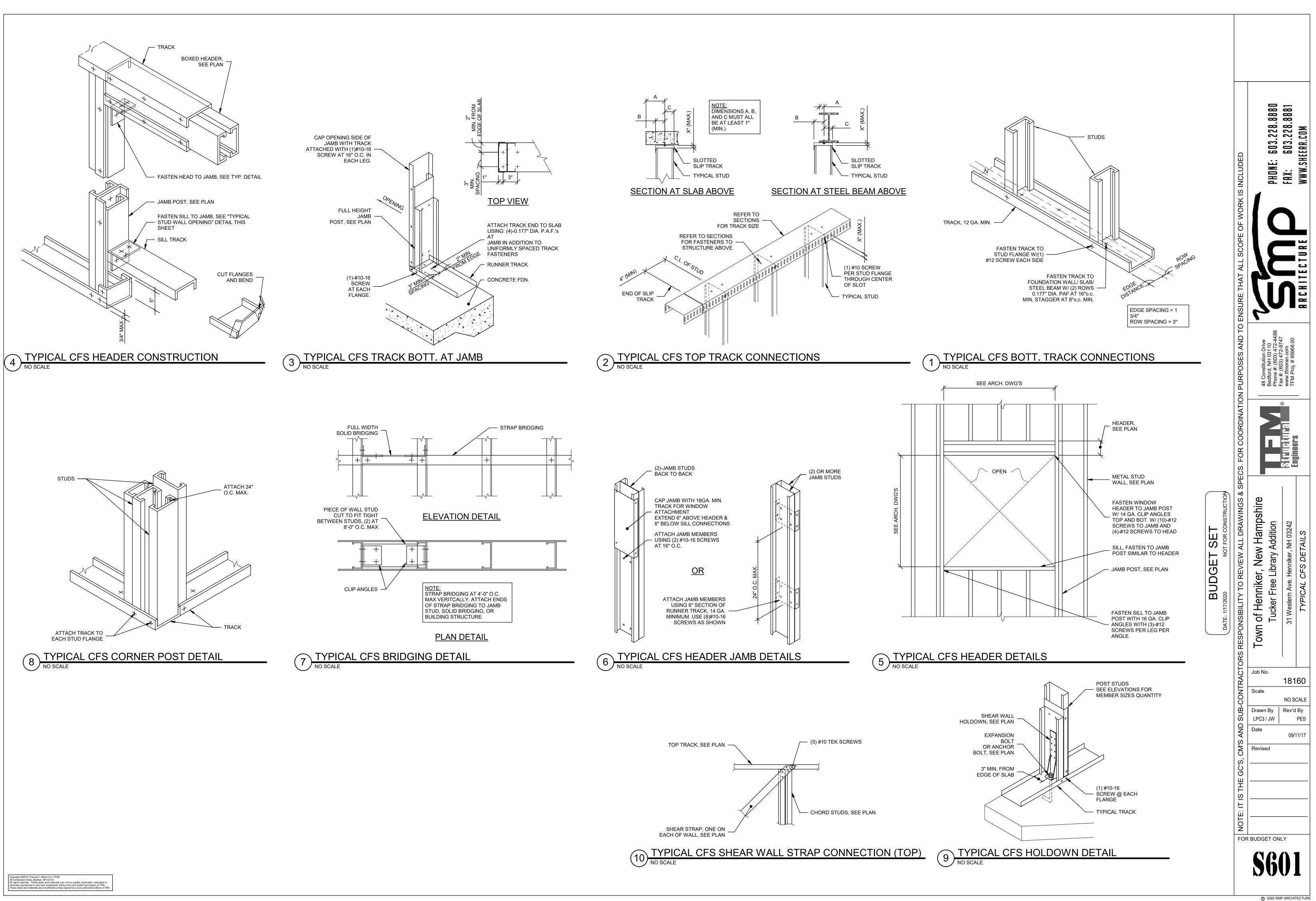
FABRICATOR SHALL DESIGN TRUSS FOR 150 PLF LATERAL IN-PLANE SHEAR FORCE

SCREW END TRUSS BOT. CHORD DN. TO TOP PL'S W/ (2)-SIMPSON SDS25450 SCREWS AT 16"o.c.

> WALL SHEATHING, SEE PLAN

WOOD STUD WALL W/ FLAT 2x6 BLOCKING BTWN. STUDS AT WALL SHEATHING PANEL EDGES, SEE PLAN





		STEEL L	INTEL SCHEDULE		
MAX RO	4" VENEER	6" WALL	8" WALL	10" WALL	12" WALL
3'-0"	(1) - L3 1/2x3 1/2x5/16	(2) - L3x2 1/2x5/16	(2) - L3 1/2x3 1/2x5/16	(1) WT 7x21.5	(3) - L4x3 1/2x5
4'-0"	(1) - L4x3 1/2x5/16	(2) - L3x2 1/2x5/16	(2) - L4x3 1/2x5/16	(1) WT 7x21.5	(3) - L5x3 1/2x5
5'-0"	(1) - L5x3 1/2x5/16 LLV	(1) WT 7x13	(2) - L5x3 1/2x5/16	(1) WT 7x21.5	(3) - L6x3 1/2x5
6'-0"	(1) - L6x3 1/2x5/16 LLV	(1) WT 7x13	(2) - L6x3 1/2x5/16	(1) WT 7x21.5	(3) - L6x3 1/2x3
7'-0"	(1) - L6x3 1/2x3/8 LLV	(1) WT 7x13	(2) - L6x3 1/2x3/8	(1) WT 7x21.5	(3) - L6x3 1/2x3

IF LINTEL IS SPECIFIED ON PLAN WITH COVER PLATE, PLATE SHALL HAVE A WIDTH 1" LESS THAN THE WALL THICKNESS. LINTELS SUPPORTING EXTERIOR MASONRY OR LOCATED IN EXTERIOR WALLS SHALL BE HOT DIPPED GALVANIZED.

CORE FILL (2) COURSES OF MASONRY BELOW BEARING WITH MORTAR AT EXISTING WALLS.

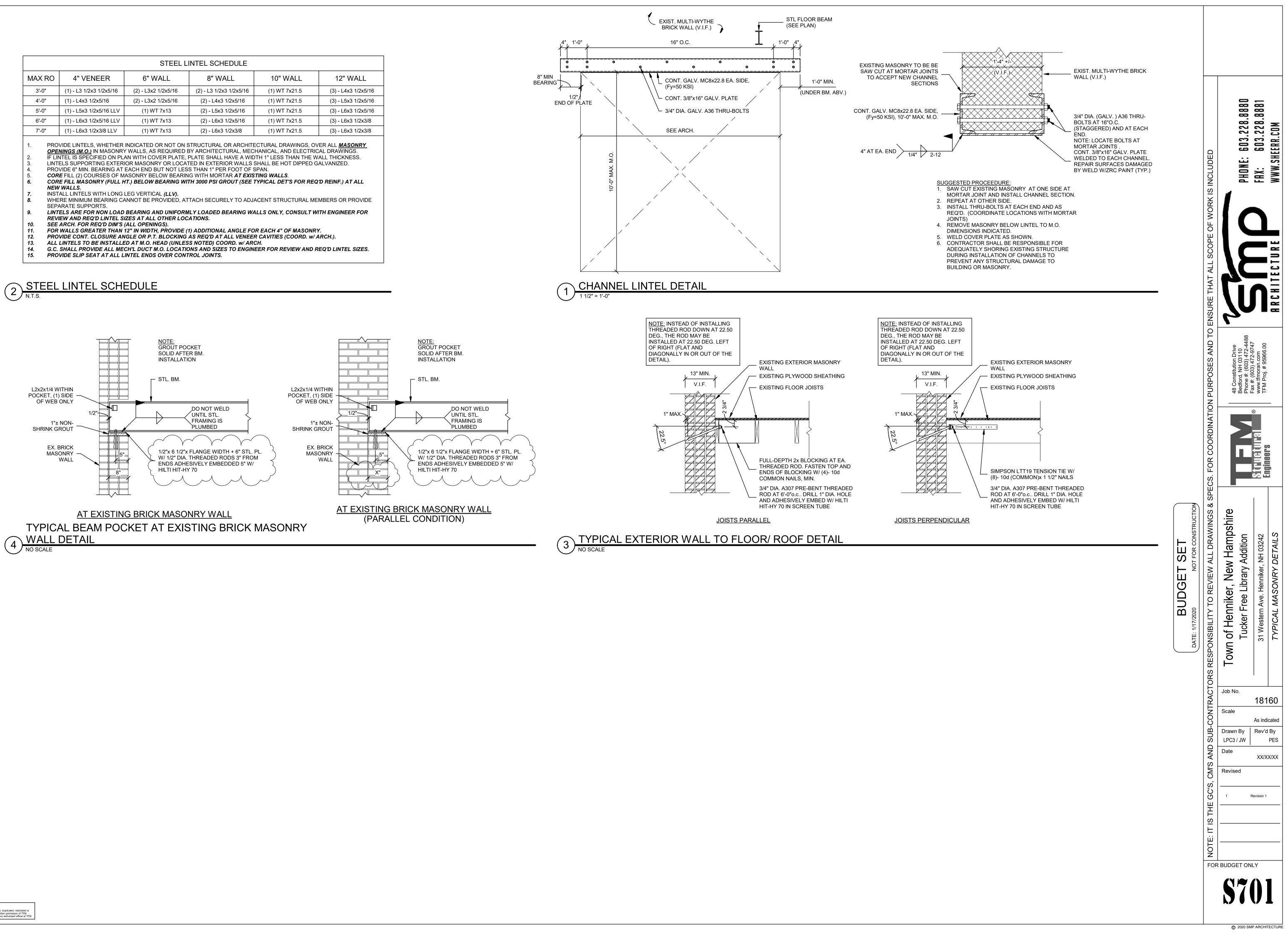
NEW WALLS.

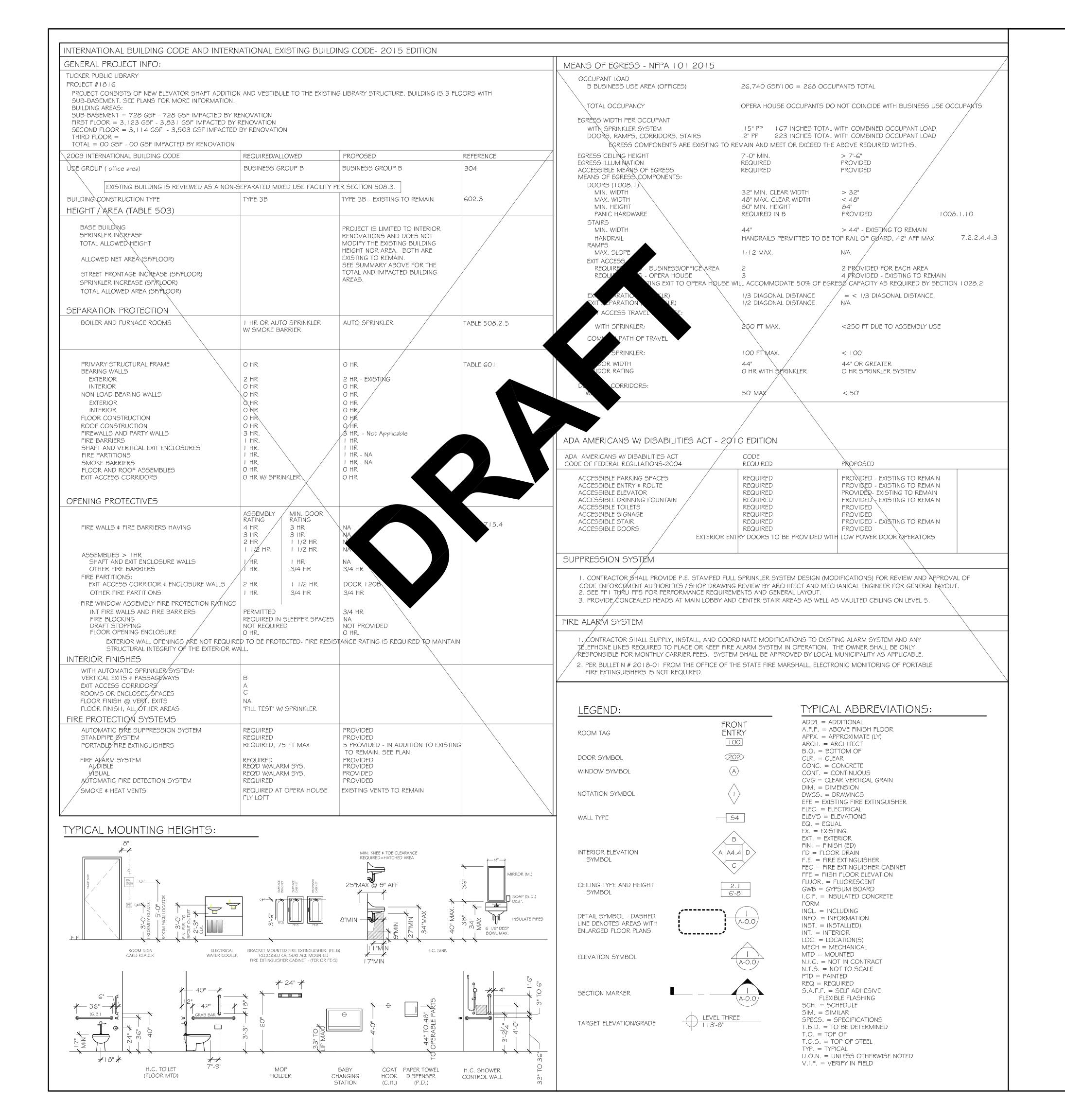
8. SEPARATE SUPPORTS.

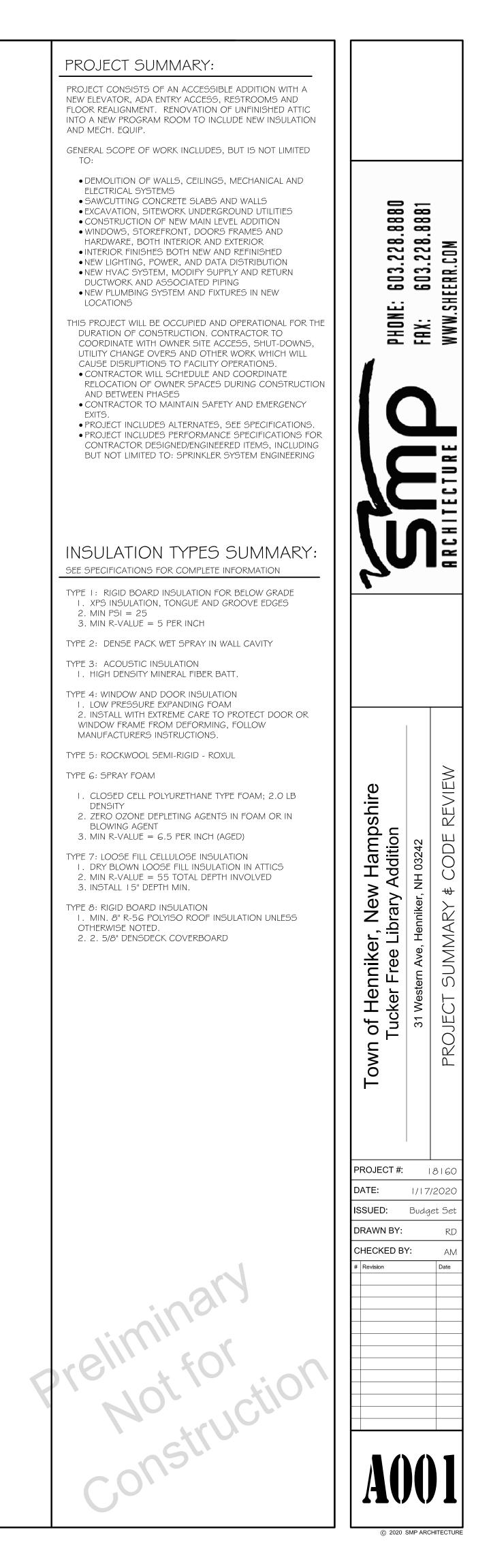
SEE ARCH. FOR REQ'D DIM'S (ALL OPENINGS).

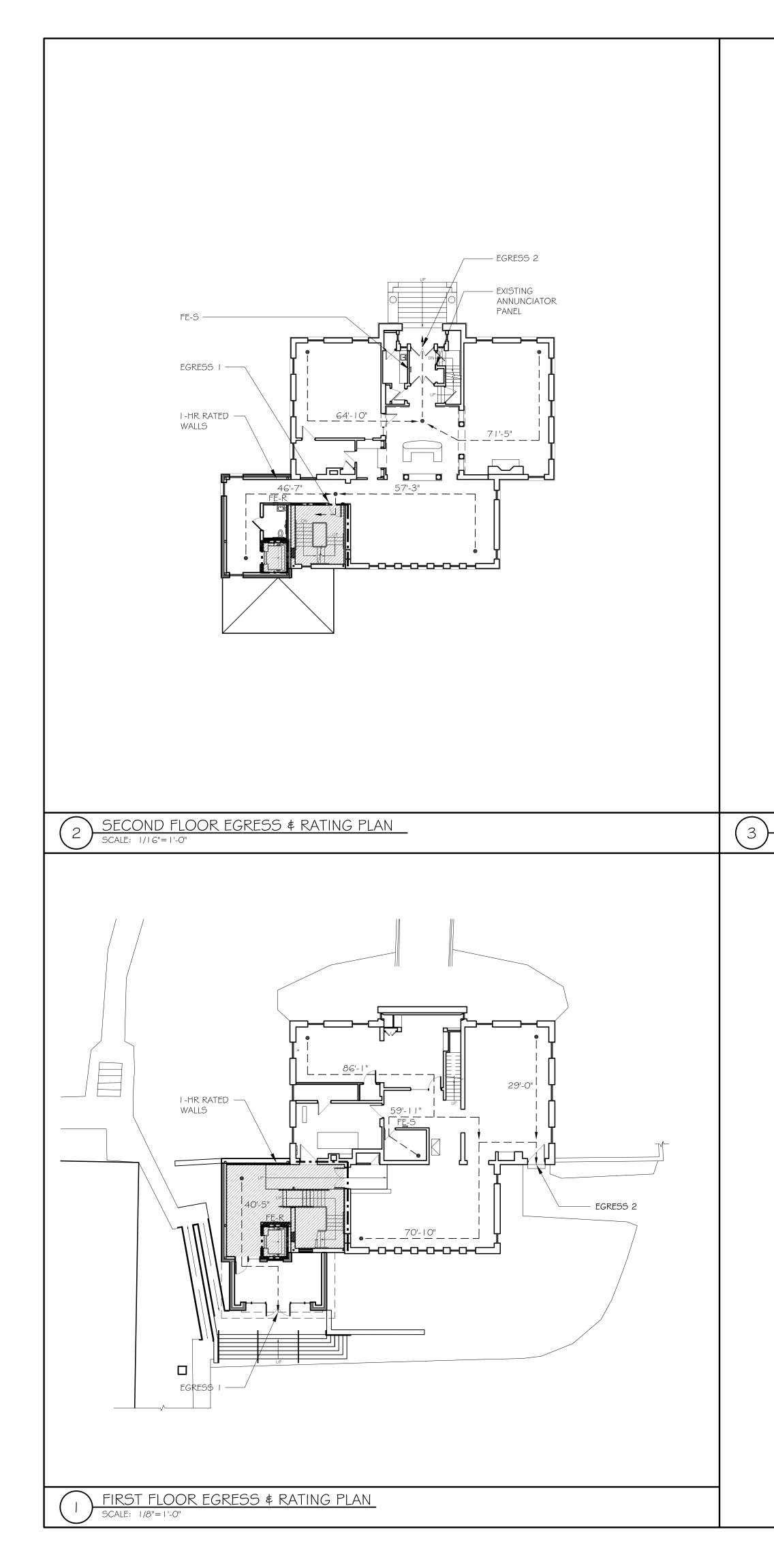
ALL LINTELS TO BE INSTALLED AT M.O. HEAD (UNLESS NOTED) COORD. w/ ARCH.

PROVIDE SLIP SEAT AT ALL LINTEL ENDS OVER CONTROL JOINTS.

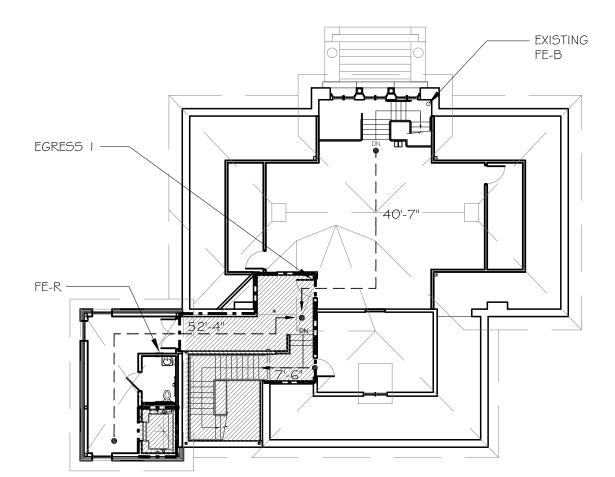








WALL TYPES



# THIRD FLOOR EGRESS & RATING PLAN SCALE: 1/16"=1'-0"

## NEW I HOUR RATED WALL

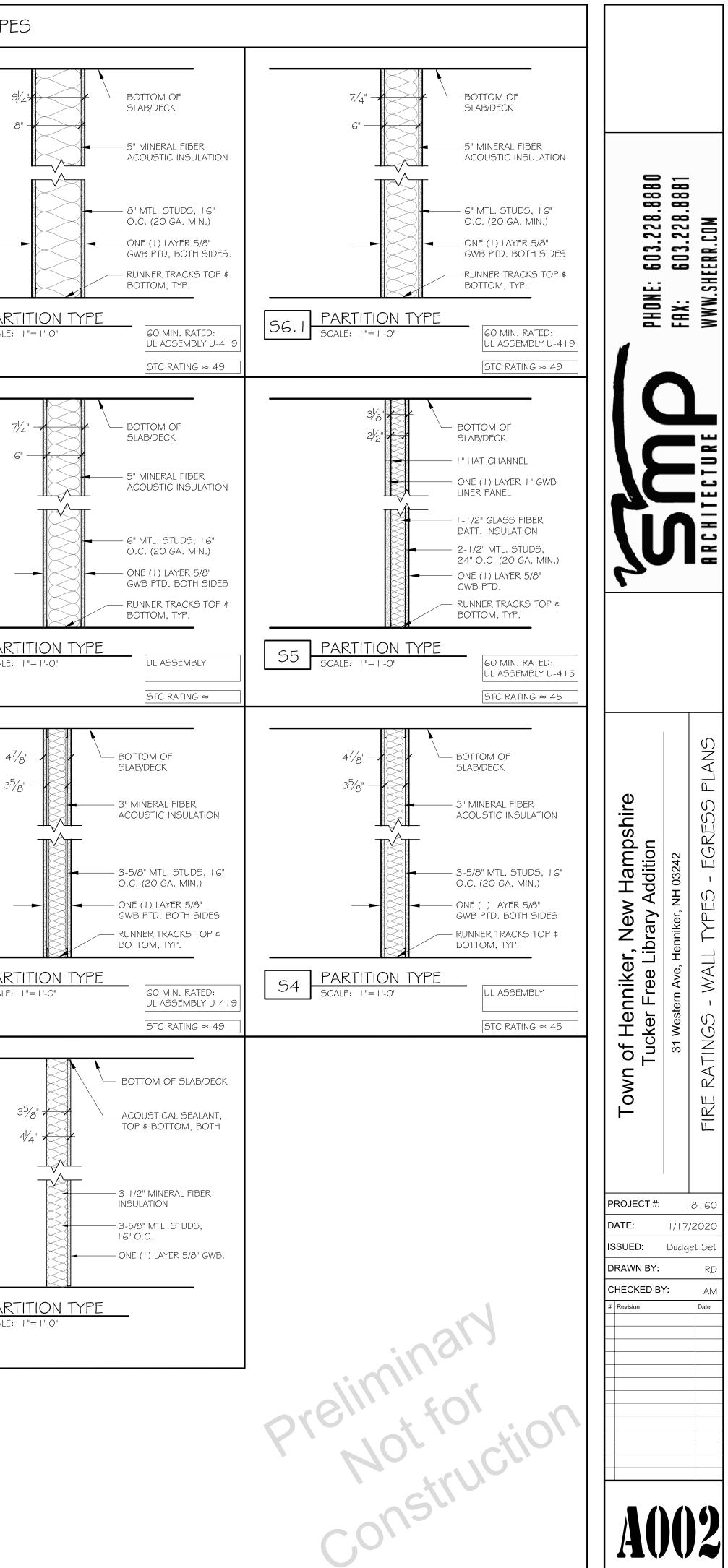
HATCH DENOTES FIRE RATED SPACE

## PARTITION NOTES:

I. SEE STRUCTURAL DRAWINGS FOR FRAMING AND SHEATHING SIZE AND INSTALLATION REQUIREMENTS.

- 2. ALL RATED WALLS & SMOKE BARRIERS TO EXTEND TO HORIZONTAL GWB ASSEMBLY OR ROOF DECK ABOVE. SEAL ALL PENETRATIONS, DECK FLUTES, ETC. AS REQ.; FIRESTOP @ RATED WALLS, FOAM INSUL. @ OTHERS.
- 3. ALL (NOTED) EXISTING FIRE RATED WALLS SHALL BE REVIEWED FOR COMPLETENESS, SEAL ALL PENETRATIONS, DECK FLUTES, ETC AS REQ.
- 4. ALL (NEW) RATED WALLS & SMOKE BARRIERS TO EXTEND TO HORIZONTAL GWB ASSEMBLY OR ROOF DECK ABOVE. SEAL ALL PENETRATIONS, DECK FLUTES, ETC. AS REQ.; FIRESTOP @ RATED WALLS , FOAM INSUL. @ OTHERS.
- 5. ALL RATED WALLS SHALL BE EFFECTIVELY ₿ PERMANENTLY IDENTIFIED BY STENCILING THE FOLLOWING: "FIRE and/or SMOKE BARRIER - PROTECT ALL OPENINGS" LETTERING SHALL BE 2" HIGH \$ STATEMENT SHALL OCCUR 30'-0" ALONG WALL. LETTERING SHALL BE PLACED @ TOP OF WALL ABOVE CEILINGS OR IN ATTIC SERVICE AREAS.
- 6. ALL REST ROOMS, SOUND ¢ OFFICE PARTITIONS TO EXTEND TO DECK ABOVE; SEAL ALL PENETRATIONS, TYP.
- 7. AT REST ROOMS ≰ BEHIND COUNTERS W/ SINKS, PROVIDE M.R. GWB. IN LIEU OF GWB. INDICATED ON PARTITION TYPE.
- 8. PROVIDE GWB CONTROL JOINTS, SPACED AS PER SPECIFICATIONS. VERIFY LOCATIONS w/ ARCHITECT IN FIELD.
- 9. PROVIDE BLOCKING AS REQ'D @ COUNTERS, SHELVES, BATHROOM FIXTURES, DISPLAY BOARDS, ETC. TYP.
- IO. SEE STRUCT DWGS FOR SHEAR WALL LOCATIONS \$ REQUIREMENTS, TYP.
- II. ALL DIMENSIONS ARE GIVEN TO FACE OF EXISTING WALL FINISH, FACE OF STUDS, FACE OF MASONRY WALL OR MASONRY OPENING.

S8.1 PA	F
SG PA SCAL	<u>F</u> _E
	4
S4.1 PA SCAL	R .E
F4 PA	R



$\langle 1 \rangle$	SAW CUT & REMOVE CONCRETE SLAB, EXCAVATE AS REQUIRED FOR NEW WORK. COORDINATE W/ RESPECTIVE DRAWINGS FOR INTENDED FINISHED RESULT.
$\langle 2 \rangle$	REMOVE STUD WALL ASSEMBLY, OR PORTION THEREOF.
$\langle 3 \rangle$	REMOVE MASONRY OR CONCRETE WALL ASSEMBLY, OR PORTION THEREOF.
$\langle 4 \rangle$	REMOVE FLOOR FINISH, PAD, TRANSITION STRIPS & MASTIC DOWN TO FINISHED SURFACE. LEAVE SURFACE SMOOTH, CLEAN & READY TO RECEIVE NEW FINISH.
$\langle 5 \rangle$	angle REMOVE FRAMING/STRUCTURE IN AREA INDICATED; COORDINATE WITH STRUCTURAL DRAWINGS.
$\langle 6 \rangle$	REMOVE STAIR ASSEMBLY. REMOVE HANDRAILS.
V GA	REMOVE EXISTING HANDRAILS ONLY. RETAIN EXISTING STAIRS.
$\langle 7 \rangle$	REMOVE WINDOW ASSEMBLY BACK TO ROUGH OPENING.
$\sqrt{7A}$	REMOVE WINDOW AND FRAME.
$\langle 8 \rangle$	angle REMOVE DOOR AND FRAME ASSEMBLY BACK TO ROUGH OPENING.
$\bigvee$	angle REMOVE CEILING FINISH AND ASSOCIATED FRAMING SYSTEM (IF HUNG CEILING TYPE); REMOVE ALL ASSOCIATED LIGHTING, DIFFUSERS AND DEVICES.
$\sim$	REMOVE ALL TOILET FIXTURES, ACCESSORIES, AND PARTITIONS.
$\checkmark$	REMOVE PLUMBING FIXTURES, SHUT OFF, DISCONNECT AND CAP ALL ASSOCIATED PIPING.
$\bigvee$	REMOVE FEMILING FIXTURES, SHOT OFF, DISCONNECT AND CAT ALL ASSOCIATED FITTING.       REMOVE DOOR, FRAME TO REMAIN
$\begin{pmatrix} & & \\ & & \\ & & \\ & & & & \\ & & & \\ & & & \\ & & & & \\ & & & \\ & & & \\ & & & \\ & & & \\ & & & \\ & & & \\ & & & \\ & & $	REMOVE ELECTRICAL FIXTURES, SHUT OFF, DISCONNECT AND CAP ALL ASSOCIATED WIRING.
$\bigvee$	
$\langle \rangle$	
$\wedge$	REMOVE AND RETAIN TRIM AND BASEBOARD. COORDINATE W/RESPECTIVE DRAWING FOR INTENDED FINISHED RESULT.
$\langle \rangle$	REMOVE EXISTING MACHINERY AND ALL ASSOCIATED MATERIALS.
$\langle  7 \rangle$	> REMOVE EXISTING HVAC AND DUCTWORK. > REMOVE WHOLE FLOOR ASSEMBLY IN E-ROOM; THIS INCLUDES WOOD JOISTS, DECK, CARPET, ETC.
	S-5-1/2" +/- S-5-1/2" +/- S-
	DEMO STONE R WALLS ON BOT REMOVE EXIST. WALKWAY.

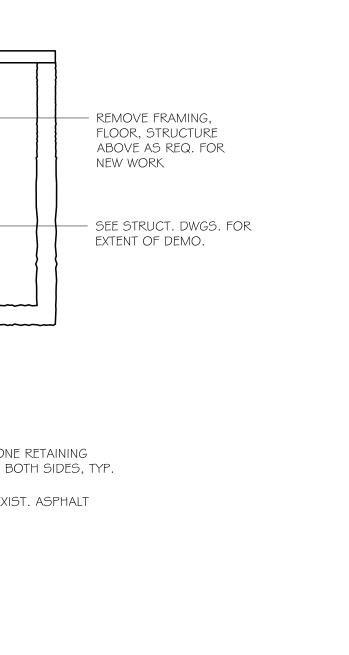
 $\leq$ SCALE: 1/8"=1'-0"

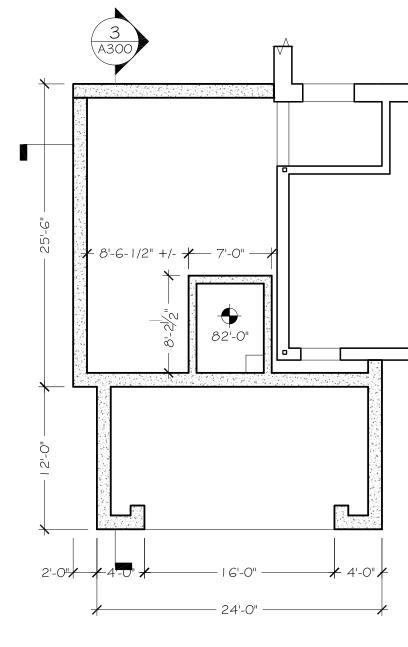
## GENERAL NOTES

- I. REFERENCE ELEVATION I OO'-O" = EXISTING LIBRARY MAIN LEVEL
- 2. SEE CIVIL PLANS FOR SITE INFORMATION.
- 3. SEE STRUCTURAL PLANS FOR STRUCTURAL DETAILS, LINTEL SIZES ≰ NOTES.
- 4. SEE MECHANICAL DRAWINGS FOR DUCT SIZES.
- 5. SEE AOO2 FOR WALL TYPES.
- 6. ALL DIMENSIONS ARE GIVEN TO FACE OF EXISTING WALL FINISH, FACE OF STUDS, FACE OF MASONRY WALL OR MASONRY OPENING, CENTER OF STRUCT. STEEL
- 7. ALL DOORS TO BE INSTALLED WITH HINGE SIDE 4" FROM NEAREST INSIDE CORNER, UNLESS OTHERWISE DIMENSIONED.
- 8. PROVIDE BLOCKING AS REQ. FOR TOILET ACCESSORIES, CABINETS, SHELVES, ETC.
- 9. PROVIDE FIREBLOCKING/FIRESTOPPING AS REQ. AT PENETRATIONS IN RATED WALLS/CEILINGS, TYP.
- 10. PROVIDE LOW PRESSURE SPRAY FOAM INSULATION AROUND ALL DOORS, WINDOWS, ETC., WHERE NECESSARY IN EXTERIOR WALLS TO MEET AIR INFILTRATION REQUIREMENTS, SEE SPECIFICATIONS. II. PROVIDE SEALANT AS REQUIRED AT TRIM, WINDOWS, ETC., TYP.
- 12. ALL PAINTS AND ADHESIVES TO BE LOW V.O.C.
- 13. PATCH TO MATCH EXISTING AS REQUIRED DUE TO NEW WORK.
- 14. ALL EXTERIOR GRADE MOUNTED MECHANICAL EQUIPMENT TO BE ON CONC. SLAB. SEE MECHANICAL, STRUCTURAL AND CIVIL DRAWINGS.
- 15. ALL EXTERIOR CONC. PADS SHALL PITCH AWAY FROM BUILDING 1/4" PER FT., UNLESS OTHERWISE NOTED.
- I G. NOTIFY ARCHITECT IMMEDIATELY REGARDING ANY QUESTIONS / CLARIFICATIONS REGARDING NOTATIONS, DETAILS, ETC. PRIOR TO INSTALLATION.

## GENERAL DEMOLITION NOTES:

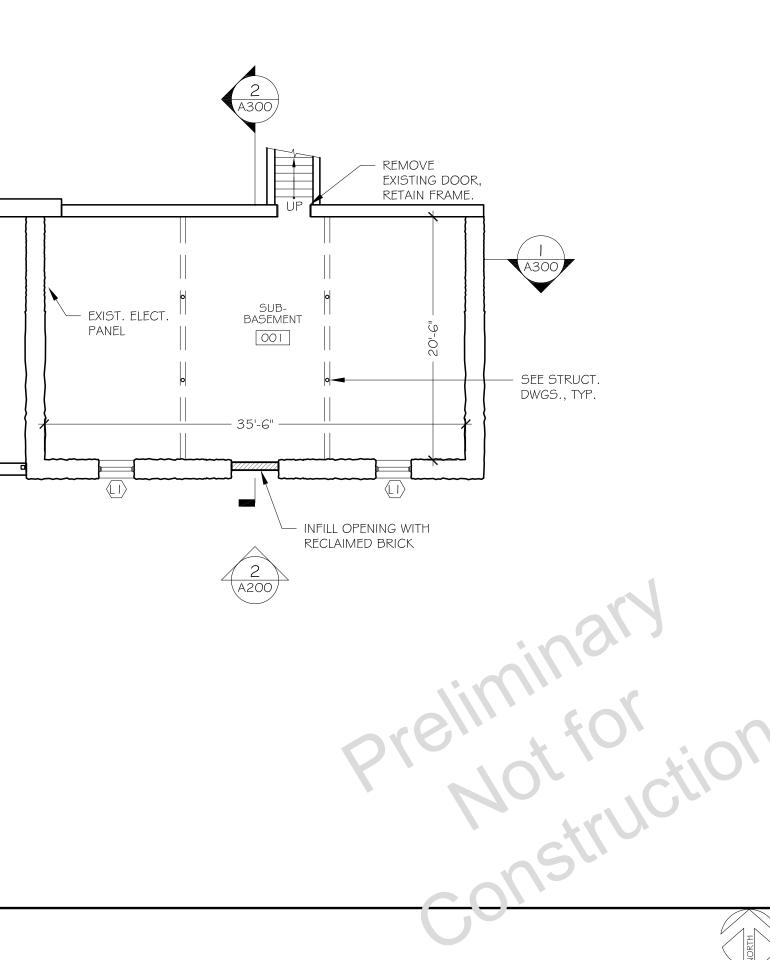
- I. OWNER WILL OCCUPY THE BUILDING FOR THE DURATION OF THE PROJECT; CONTRACTOR SHALL PHASE AND SEPARATE THE WORK TO MEET OWNERS OPERATIONAL AND SAFETY REQUIREMENTS.
- 2. CONTRACTOR SHALL PROVIDE MEANS TO MAINTAIN NEGATIVE PRESSURE IN OCCUPIED BUILDING @ ALL TIMES. 3. THE CONTRACTOR SHALL BE RESPONSIBLE FOR VERIFYING ALL EXG. CONDITIONS IN THE FIELD PRIOR TO ANY DEMOLITION OR CONSTRUCTION. ANY DISCREPANCIES RELATING TO THE DWG'S SHALL BE REPORTED TO THE ARCHITECT IMMEDIATELY.
- 4. THE CONTRACTOR SHALL BE AWARE OF SELECTIVE DEMOLITION @ ALL SECTIONS OF WORK, REVIEWING ALL NEW & RENOVATION DETAILS TO DETERMINE WHAT IS TO BE REMOVED OR TO REMAIN & WILL BE RESPONSIBLE FOR REPLACEMENT IN- KIND ALL WORK INADVERTENTLY REMOVED.
- CASEWORK OR MILLWORK SCHEDULED TO REMAIN. THE WORK AREA WILL BE LEFT CLEAN ≰ READY TO RECEIVE NEW WORK. 6. CONTRACTOR SHALL COORDINATE DEMOLITION REQUIREMENTS WITH THAT OF NEW WORK AND EQUIPMENT TO ENSURE NEW EQUIPMENT CAN BE MOVED INTO INTENDED FINAL POSITION. ITEM NOT INDICATED SPECIFICALLY INDICATED ON THE DEMOLITION DRAWINGS, BUT REQUIRED FOR INSTALLATION OF NEW WORK SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR.
- 7. CONTRACTOR SHALL BE RESPONSIBLE FOR REMOVING, RECYCLING & LEGALLY DISPOSING OF ALL MATERIALS. 8. PATCH & REPAIR ALL EXG. TO REMAIN WALLS, FLOORING, & CEILINGS DAMAGED DURING DEMOLITION OR REMOVAL OF EXG. CONSTRUCTION. REFER TO ARCHITECTURAL DRAWINGS FOR OTHER AREAS OF PATCHING, REPAIR & INFILL.
- 9. PROVIDE ROUGH OPENINGS AS REQ'D FOR NEW MECHANICAL, ELECTRICAL, OR PLUMBING COORDINATE W/ M, E, & P DWG'S. IO. SEE MECHANICAL, PLUMBING ≰ ELECTRICAL DWG'S FOR ADDITIONAL DEMO INFORMATION.
- 11. EXG. EQUIPMENT DESIGNATED TO BE RE-USED SHALL BE CAREFULLY REMOVED, ADEQUATELY STORED, & RE-INSTALLED ACCORDING TO DWG'S. COORDINATE ANY TEMPORARY CAPPING OF WATER & UTILITY LINES AS REQ'D.
- 12. STRUCTURAL STEEL ENCOUNTERED TO REMAIN. COORDINATE WITH ENGINEER AND ARCHITECT.

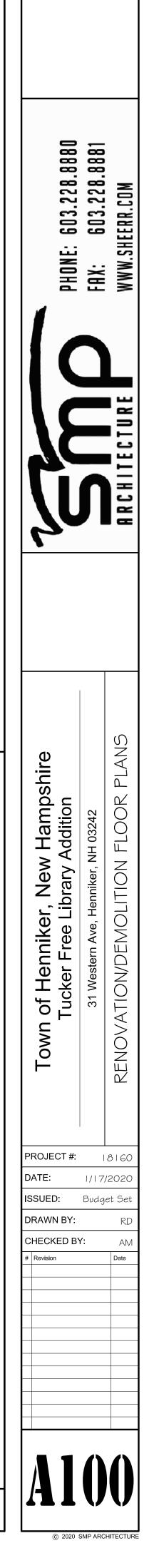


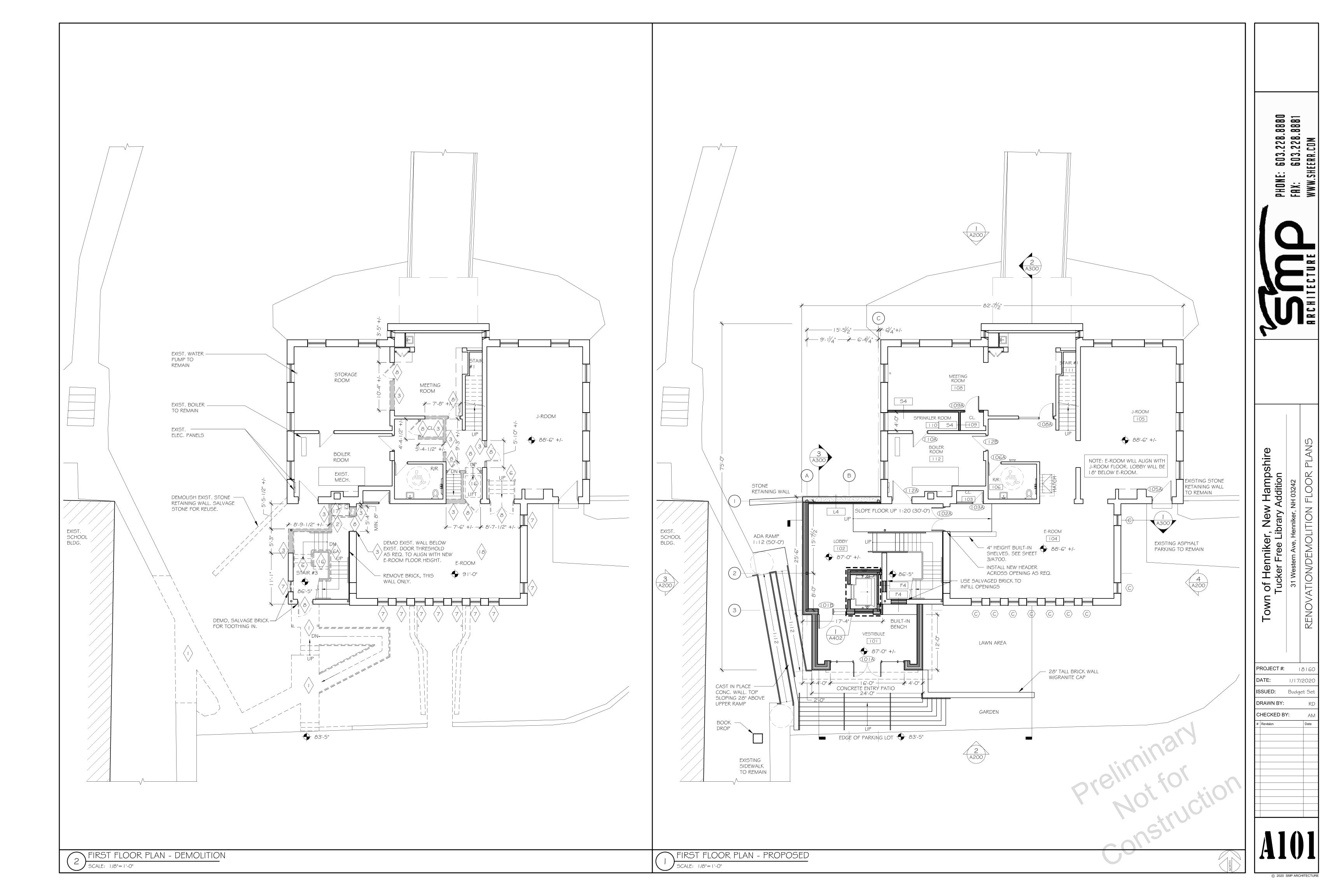


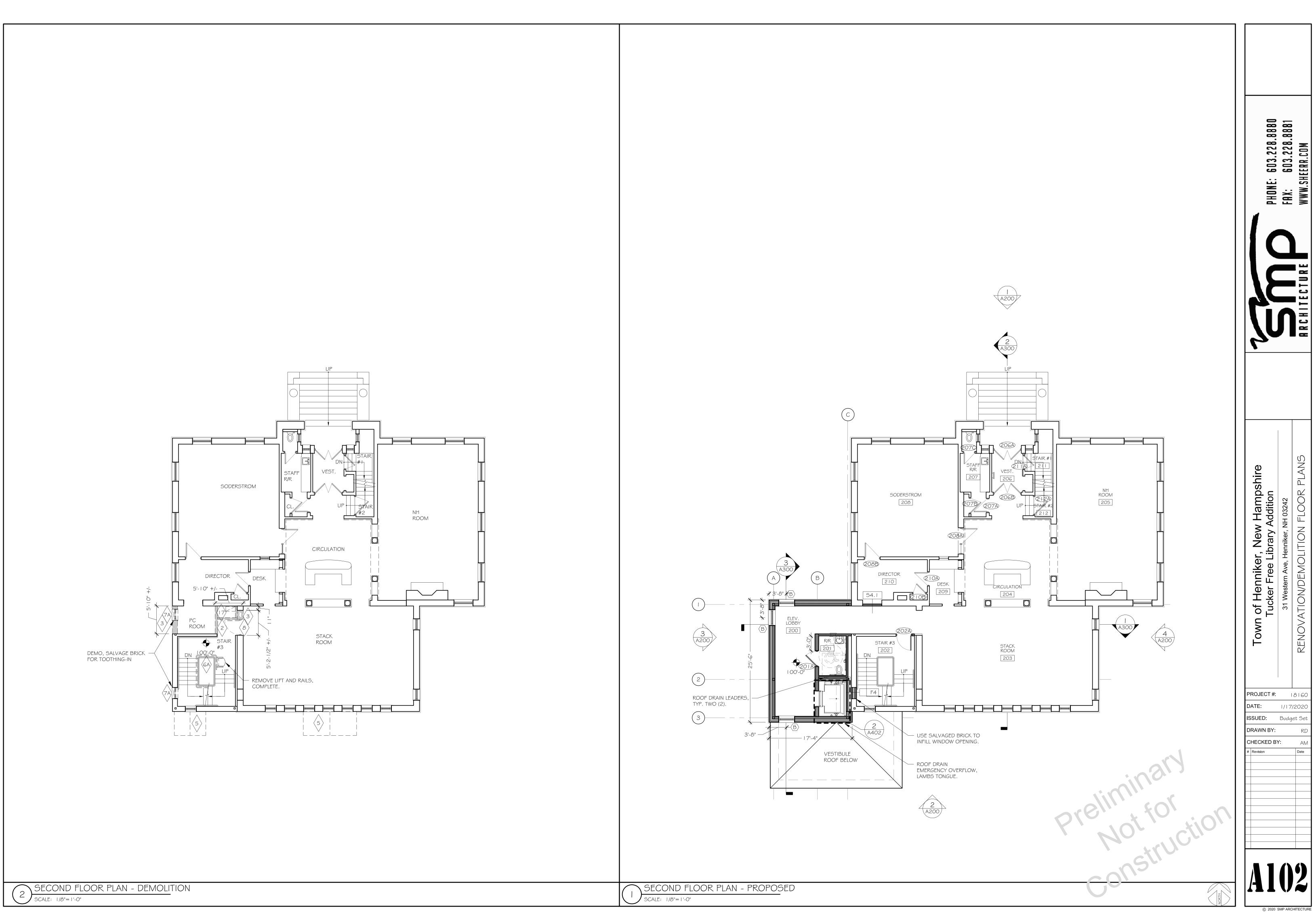
FOUNDATION/SUB-BASEMENT PLAN - PROPOSED
SCALE: 1/8"=1'-0"

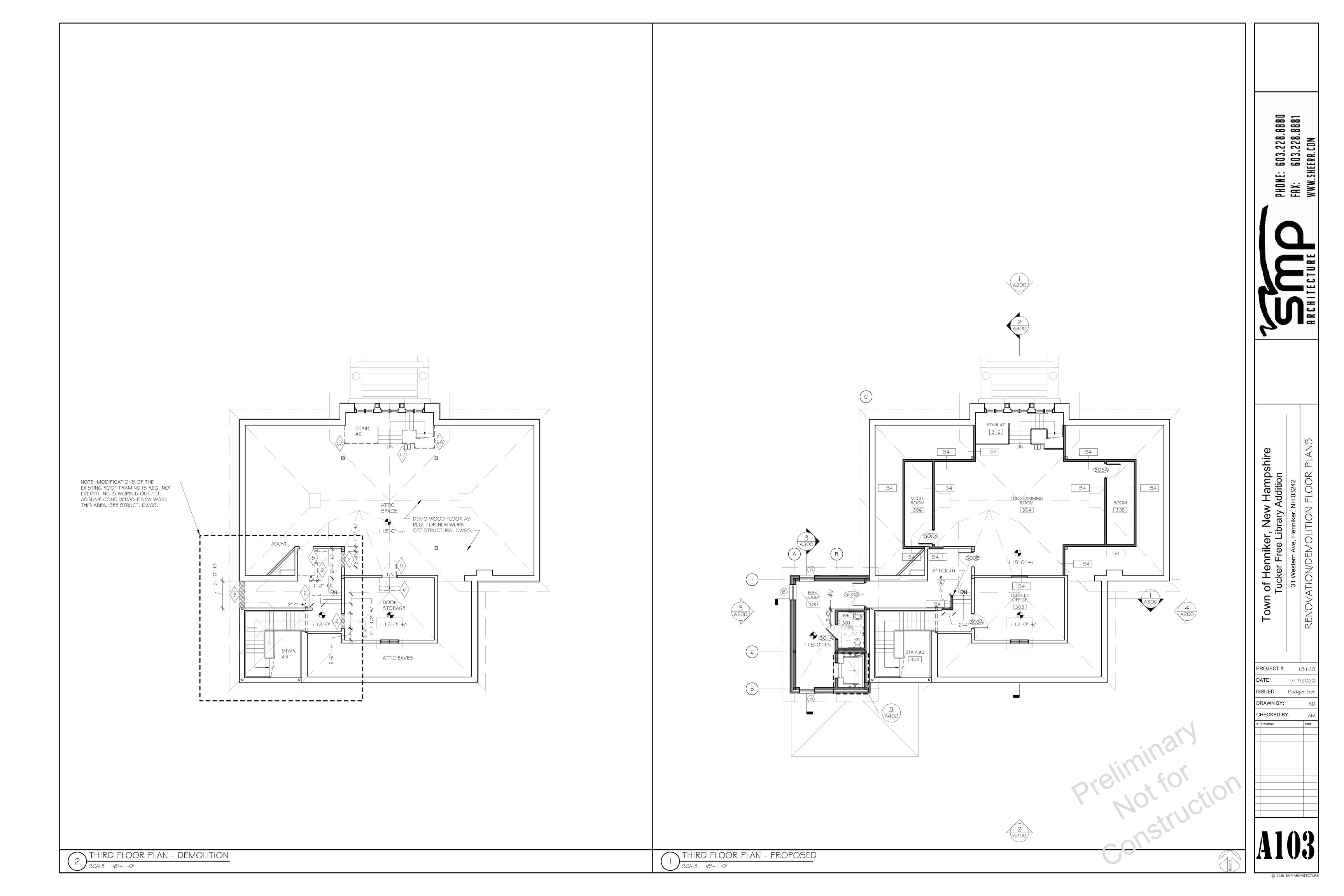
5. THE CONTRACTOR SHALL REMOVE ITEMS TO BE DEMOLISHED AS INDICATED ON THE DWG'S W/ CARE BEING TAKEN NOT TO DAMAGE ADJACENT WALLS, CEILINGS, FLOORS, FINISHES,

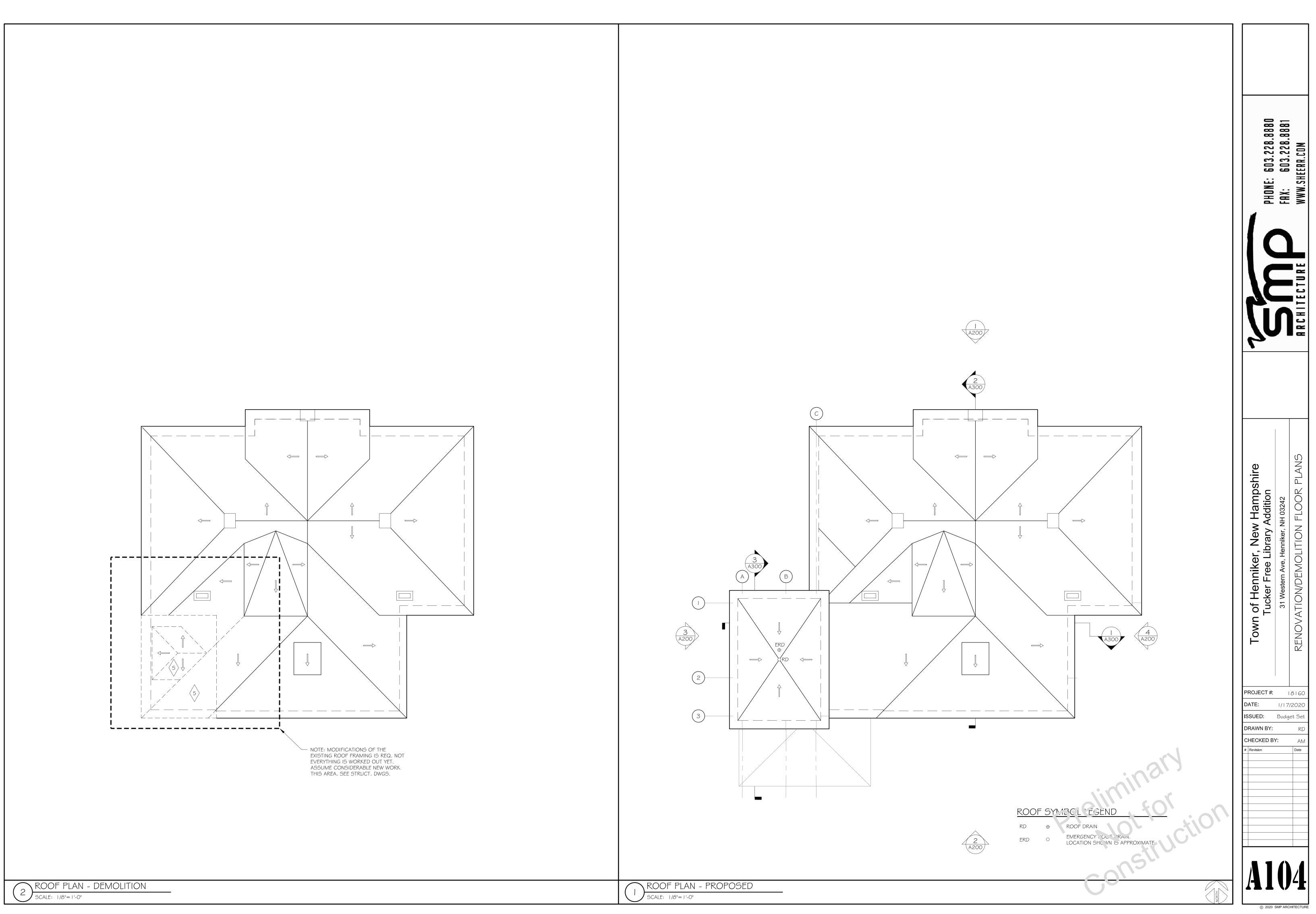




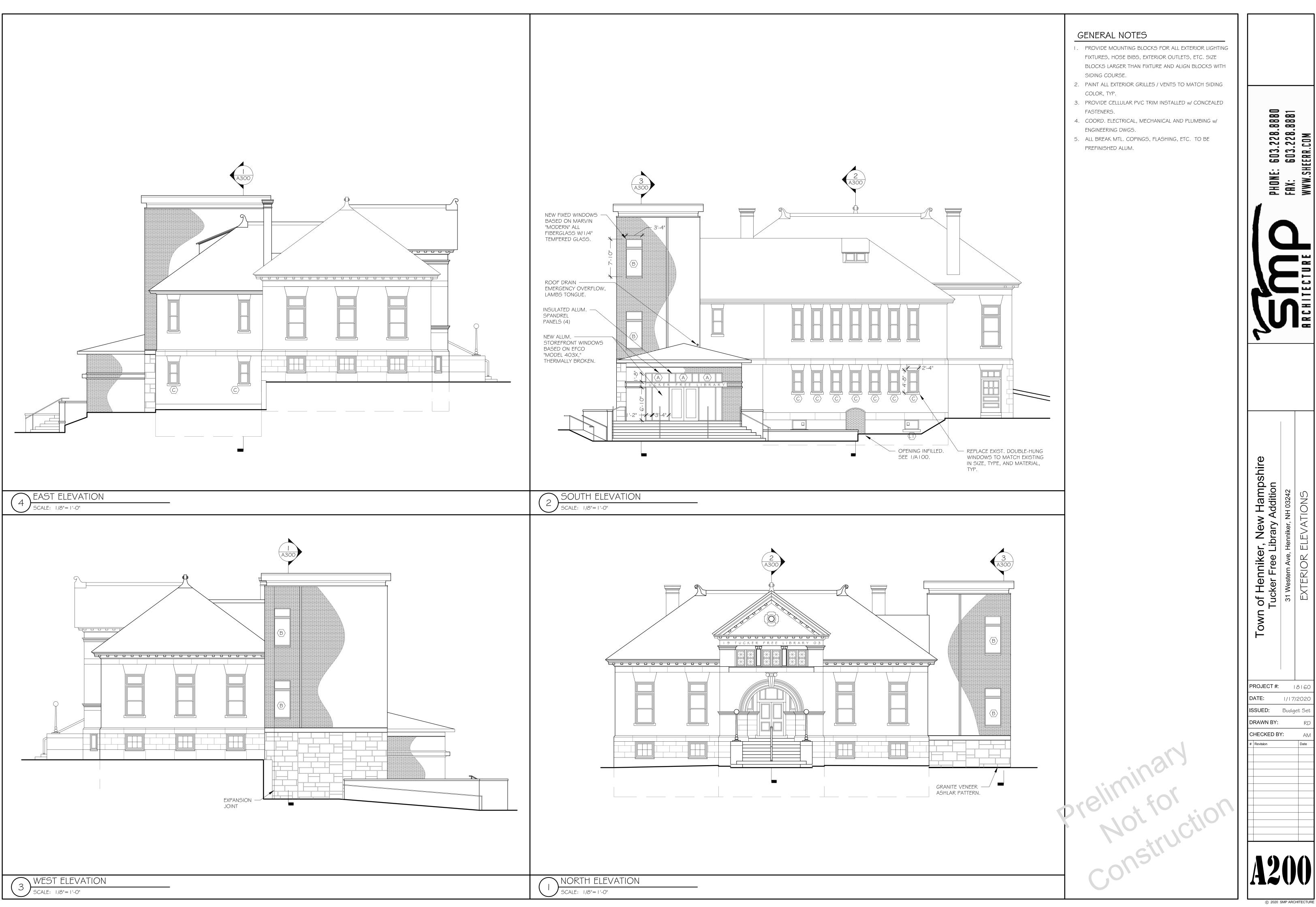












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